

TRANSPORTATION ANALYSIS

RESTORATION BAY

Prepared For

TTGC, LLC

Prepared By



*LINCKS & ASSOCIATES, INC.
Engineers – Planners
Tampa, Florida*

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Tampa, Florida 33607
813-289-0039

State of Florida Authorization No. EB0004638

November, 2019

Project No. 19145

Steven J. Henry, P.E.
P.E. No. 51555

Date



LINCKS & ASSOCIATES, INC.

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INTRODUCTION

The purpose of this report is to provide the Transportation Analysis in conjunction with the rezoning of the subject property located west of 113th Street and south of 66th Avenue North in Pinellas County, Florida as shown in Figure 1. The subject property is proposed to be rezoned to allow up to 273 Single Family Homes. The existing use of the property is a Golf Course. The access for the project is proposed to be based on two options as follows:

- Option A: One (1) full access to 66th Avenue North
- Option B: One (1) full access to 66th Avenue North and one (1) full access to Irving Avenue

This report will evaluate the two access options to serve the project.

ESTIMATED DAILY PROJECT TRAFFIC

The trip rates utilized in this report were obtained in the latest computerized version of "OTISS" which is utilizes the Institute of Transportation Engineers' (ITE) Trip Generation Manual, 10th Edition, 2017, as its data base. Based on these trip rates, it is estimated the proposed land use would generate 2,619 daily trip ends, as shown in Table 1.

ESTIMATED AM PEAK HOUR PROJECT TRAFFIC

Based on data contained in the ITE Trip Generation Manual, 10th Edition the proposed land use would generate 199 trip ends during the AM peak hour with 50 inbound and 149 outbound, as shown in Table 1.





FIGURE 1

PROJECT LOCATION

TABLE 1
TRIP GENERATION COMPARISON (1)

<u>Land Use</u>	<u>Land Use</u>	<u>Size</u>	Daily <u>Trip Ends (1)</u>	AM Peak Hour <u>Trip Ends (1)</u>		PM Peak Hour <u>Trip Ends (1)</u>	
				<u>In</u>	<u>Out</u>	<u>In</u>	<u>Out</u>
Existing	Golf Course	18 Holes	547	25	7	32	28
Proposed	Single Family	273 DU's	<u>2,619</u>	<u>50</u>	<u>149</u>	<u>199</u>	<u>168</u>
	Difference		2,072	25	142	167	98
					140	74	<u>214</u>

(1) Source: ITE Trip Generation Manual, 10th Edition, 2017.



ESTIMATED PM PEAK HOUR PROJECT TRAFFIC

Again, based on data contained in the ITE Trip Generation Manual, 10th Edition, during the PM peak hour, the proposed land use would generate 266 trip ends with 168 inbound and 98 outbound, as shown in Table 1.

PROJECT TRAFFIC DISTRIBUTION

The following distribution of the project traffic was estimated based on existing travel patterns and existing development in the vicinity of the project:

- 40% to and from the North (via 113th Street and 116th Street)
- 60% to and from the South (via 113th Street)

Table 2 provides the project traffic distribution. Figures 2 and 3 illustrates the assignment of the AM and PM peak hour project trip ends for Options A and B respectively.

BUDGETED IMPROVEMENTS

As stated previously the project is located west of 113th Street and South of 66th Avenue North. 66th Avenue North is a two lane undivided roadway and 113th Street is a six lane divided roadway in the vicinity of the project.

According to the Pinellas County Capital Improvement Plan (CIP), there are no capacity adding improvements budgeted for construction in the vicinity of the project.



TABLE 2
PROJECT TRAFFIC DISTRIBUTION

Time Period	North		South		(60%)		Total
	<u>In</u>	<u>Out</u>	<u>In</u>	<u>Out</u>	<u>In</u>	<u>Out</u>	
AM	20	60	30	89	50	149	
PM	67	39	101	59	168	98	





LEGEND

12/23 = AM/PM PEAK HOUR TRAFFIC

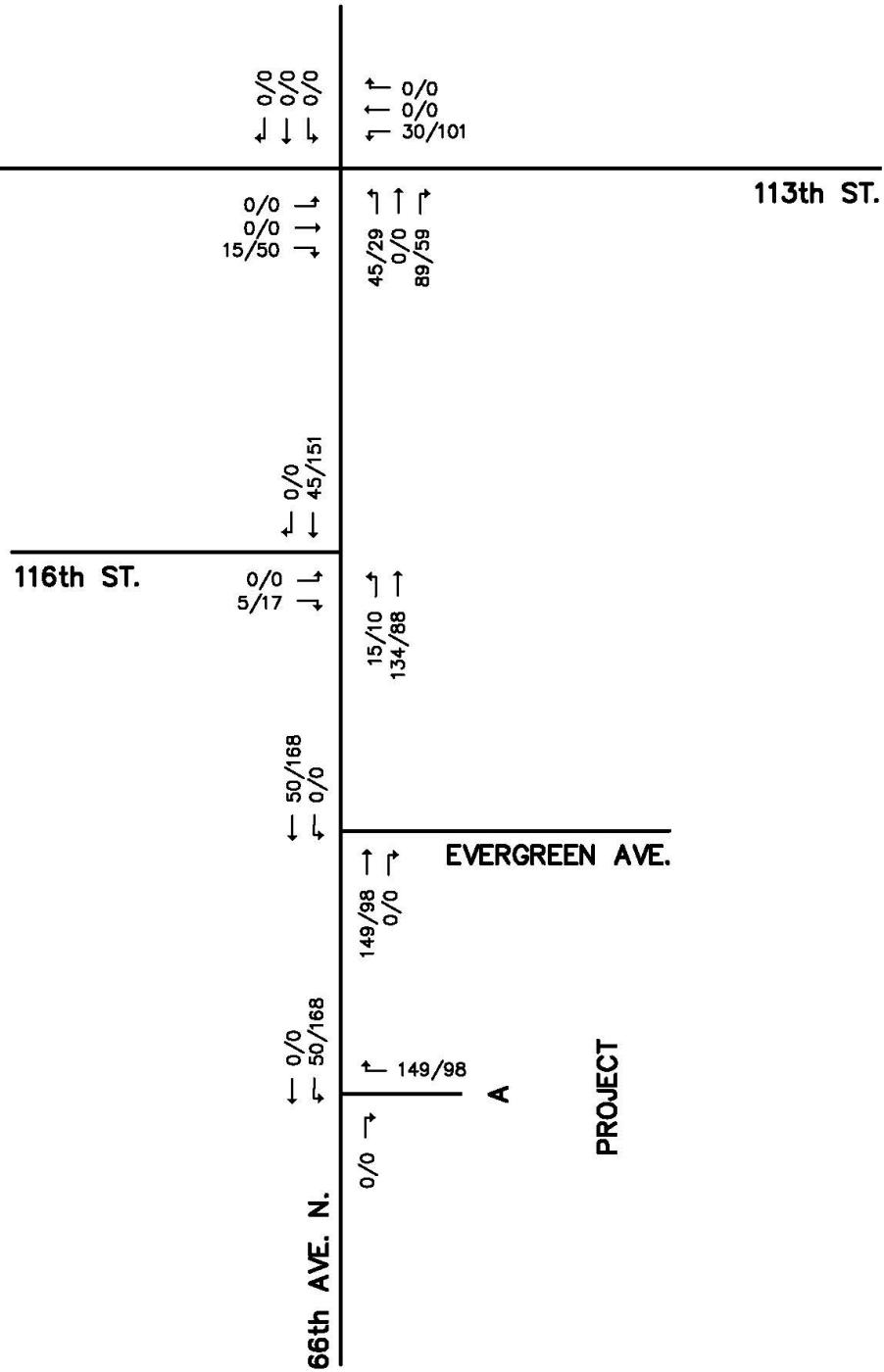


FIGURE 2
OPTION A
PROJECT TRAFFIC



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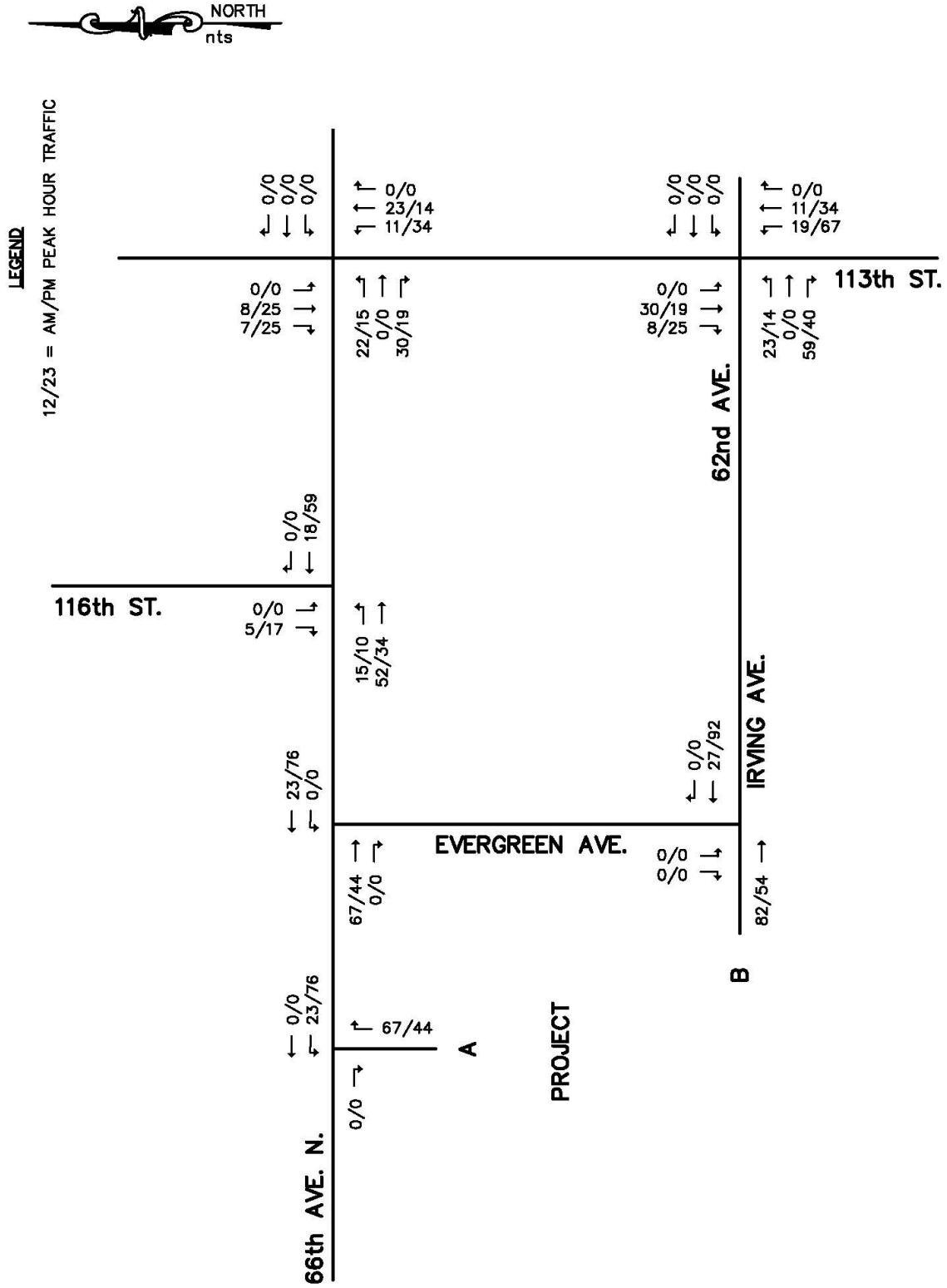


FIGURE 3
OPTION B
PROJECT TRAFFIC



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PEAK SEASON TRAFFIC

The following methodology was utilized to estimate the peak season traffic utilized in this analysis:

1. AM (7:00 – 9:00) and PM (4:00-6:00) peak hour turning movement counts were conducted at the following intersections:

- 113th Street and 66th Avenue North
- 113th Street and 62nd Avenue
- 116th Street and 66th Avenue North
- Evergreen Avenue and 66th Avenue North
- Evergreen Avenue and Irving Avenue

Figure 4 illustrates the existing counts.

2. The existing counts were adjusted to peak season based on the FDOT Seasonal Adjustment Factors for Pinellas County.

Figure 5 illustrates the peak season traffic. Figures 6 and 7 illustrates the peak season plus project traffic for the AM and PM peak hours for Options A and B respectively.

INTERSECTION ANALYSIS

Intersection analysis was conducted for the AM and PM peak hours for the Options A and B. These calculations were performed utilizing the methodology described in Chapters 18 and 19, Signalized and Unsignalized Intersections, of the Transportation Research Board (TRB) Special Report, the 2010 Highway Capacity Manual. Existing signal timings were used in the analysis. The results of the analysis are described in the following paragraphs for



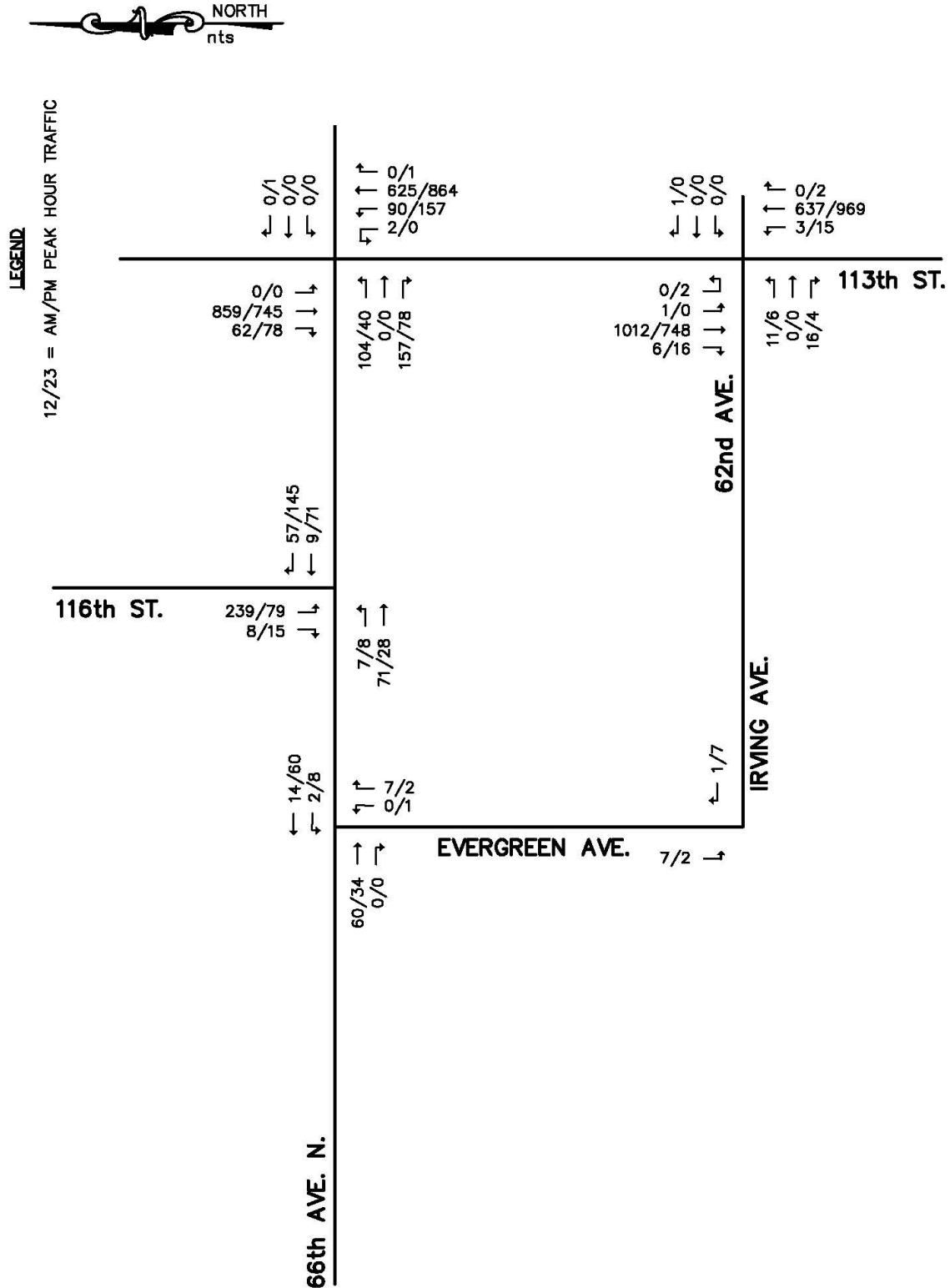
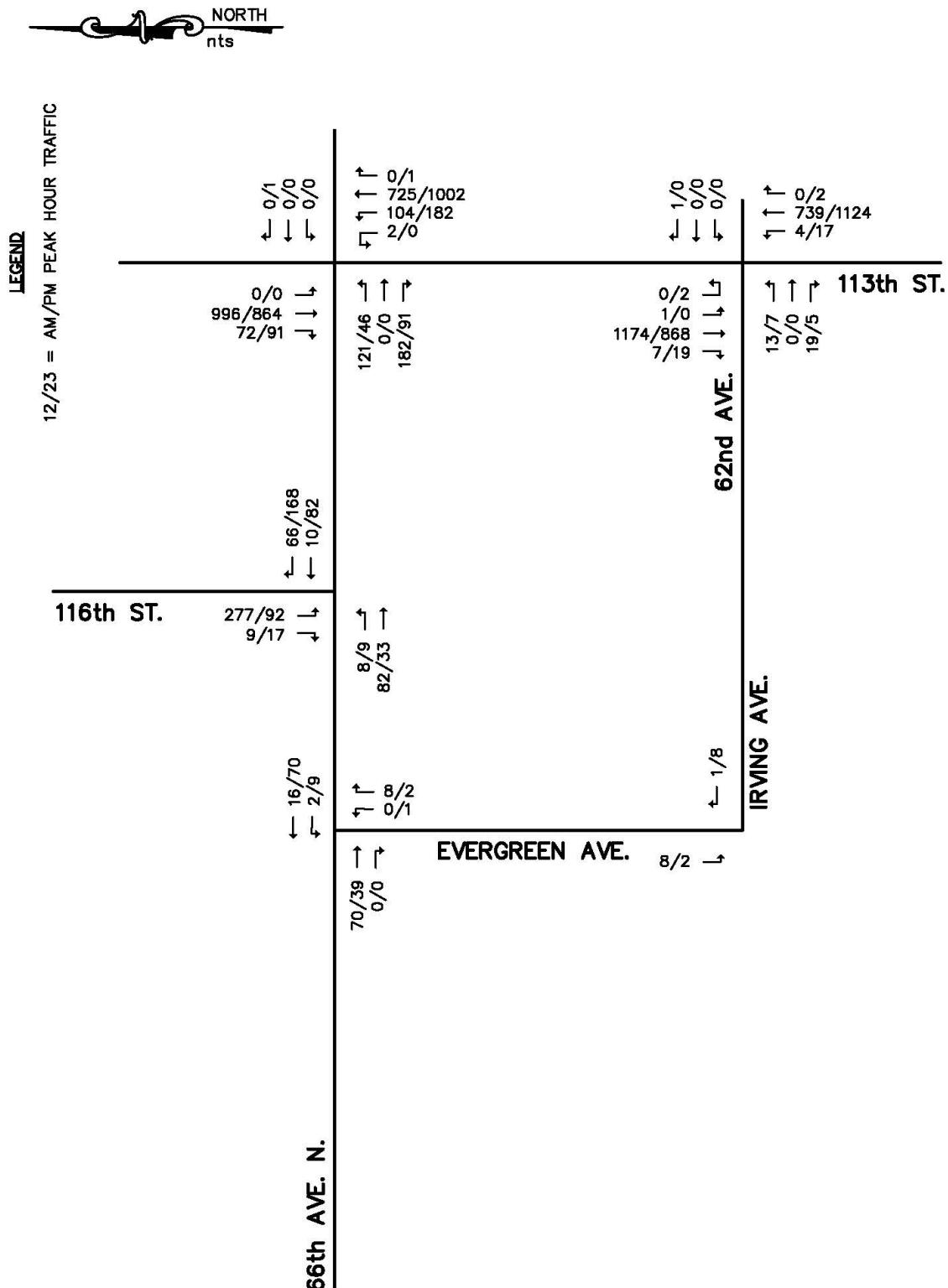


FIGURE 4
EXISTING TRAFFIC



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FIGURE 5
PEAK SEASON TRAFFIC



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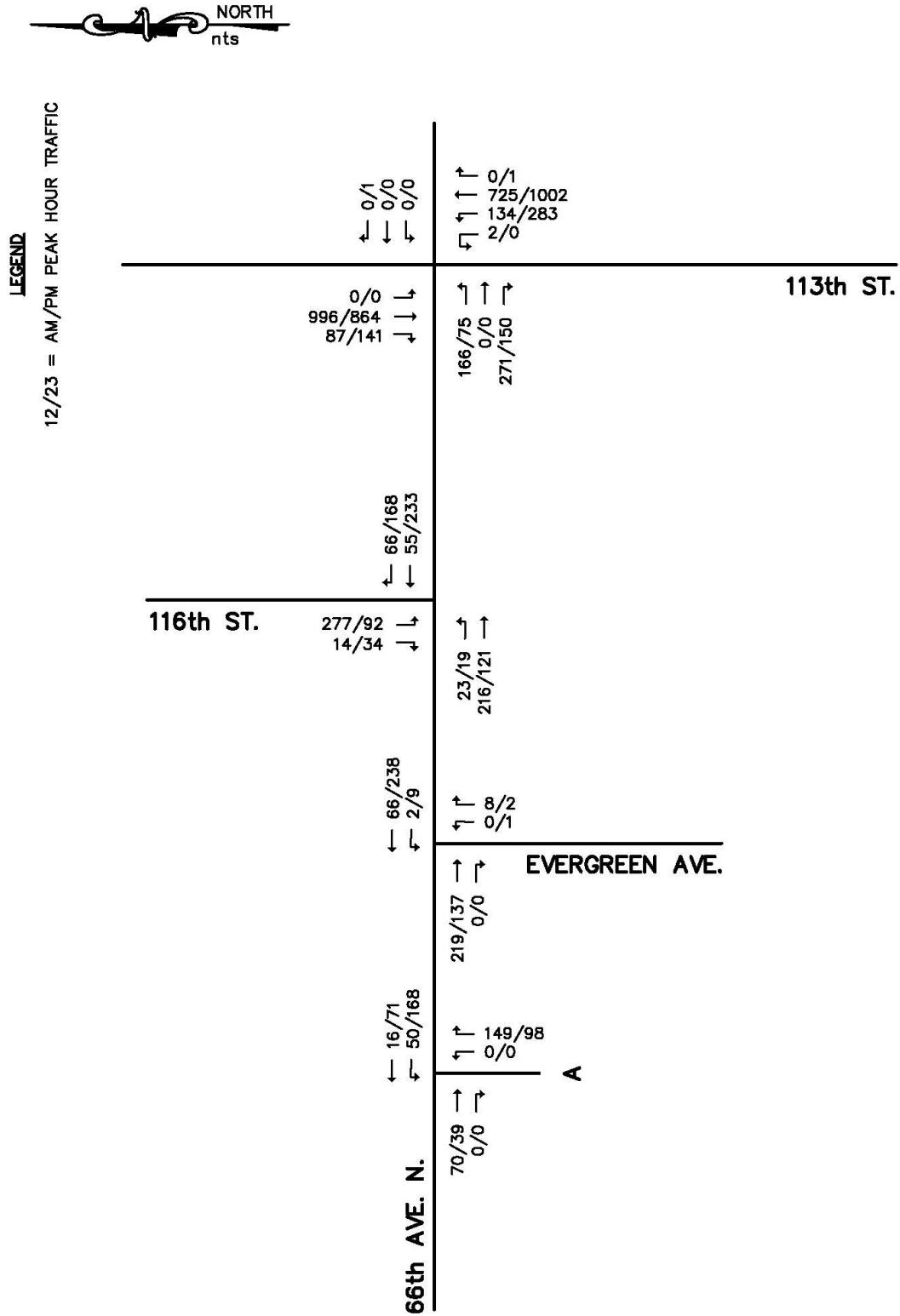
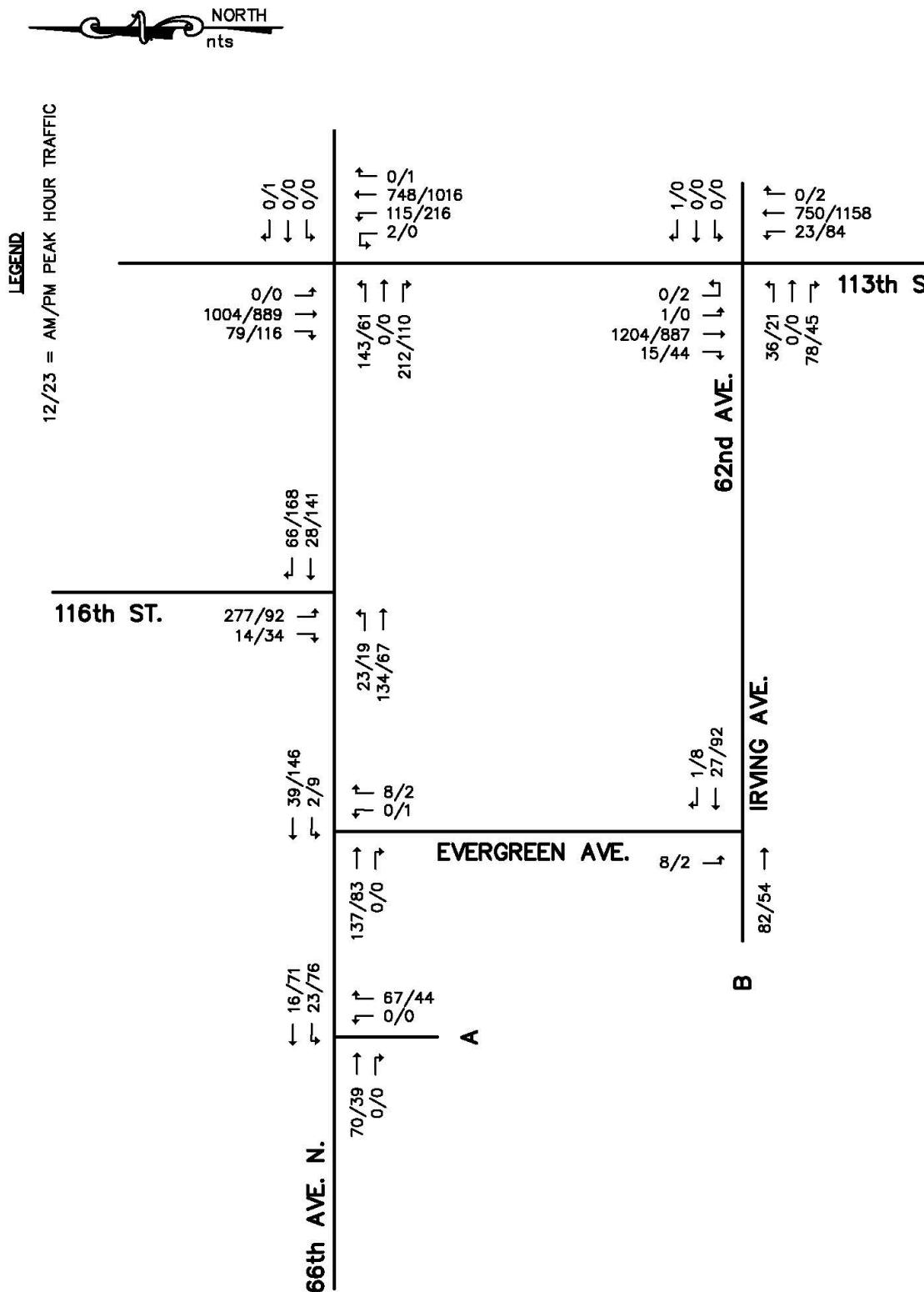


FIGURE 6
OPTION A
PEAK SEASON PLUS
PROJECT TRAFFIC





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Options A and B.

Option A

The following intersections were analyzed for the Option A:

- 113th Street and 66th Avenue North
- 116th Street and 66th Avenue North
- Project Access A and 66th Avenue North

Tables 3 and 4 summarizes the results of the analysis for the Option A and described in the following paragraphs:

113th Street and 66th Avenue North

This intersection is currently signalized. Based on signalized intersection analysis this intersection should operate at an acceptable level of service during the AM and PM peak hours with the peak season traffic and with peak season plus project traffic, as shown in Table 3.

116th Street and 66th Avenue North

This intersection is currently unsignalized. Based on unsignalized intersection analysis all movements should operate at an acceptable level of service during AM and PM peak hours with peak season traffic and with peak season plus project traffic, as shown in Table 4.

Project Access A and 66th Avenue North

This project access is proposed to have full access to 66th Avenue North. Based on unsignalized intersection analysis all movements should operate at an acceptable level of service during AM and PM peak hours with peak season plus project traffic, as shown in Table 4.



TABLE 3
OPTION A
ESTIMATED INTERSECTION LEVEL OF SERVICE
(SIGNALIZED)

Intersection	Time Period	Peak Season		Peak Season Plus	
		Traffic	Project Traffic	Traffic	Project Traffic
113th Street and 66th Avenue North	AM PM	B A	C B	C B	C B



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TABLE 4
OPTION A
ESTIMATED INTERSECTION LEVEL OF SERVICE
(UN SIGNALIZED)

Intersection	Direction	AM Peak Hour				PM Peak Hour				Peak Season Plus			
		Peak Season		Project Traffic		Peak Season Plus		Project Traffic		Peak Season		Project Traffic	
		Left Delay	Through Delay	Right Delay	Left Delay	Through Delay	Right Delay	Left Delay	Through Delay	Right Delay	Left Delay	Through Delay	Right Delay
116th Street and 66th Avenue North	EB	9.1	9.1	-	13.0	13.0	-	7.7	7.7	-	8.8	8.8	-
	WB	-	8.2	8.2	-	9.9	9.9	-	8.3	8.3	-	11.0	11.0
	SB	12.1	-	12.1	15.5	-	15.5	8.4	-	8.4	9.3	-	9.3
Project Access A and 66th Avenue North	WB	-	-	-	7.5	7.5	-	-	-	-	7.8	7.8	-
	NB	-	-	-	9.5	-	9.5	-	-	-	9.2	-	9.2



Option B

The following intersections were analyzed for the Option B:

- 113th Street and 66th Avenue North
- 116th Street and 66th Avenue North
- 113th Street and 62nd Avenue
- Project Access A and 66th Avenue North
- Project Access B and Evergreen Avenue

Tables 5 and 6 summarizes the results of the analysis for the Option B and described in the following paragraphs:

113th Street and 66th Avenue North

This intersection is currently signalized. Based on signalized intersection analysis this intersection should operate at an acceptable level of service during AM and PM peak hours with peak season traffic and with peak season plus project traffic, as shown in Table 5.

116th Street and 66th Avenue North

This intersection is currently unsignalized. Based on unsignalized intersection analysis all movements should operate at an acceptable level of service during AM and PM peak hours with peak season traffic and with peak season plus project traffic, as shown in Table 6.

113th Street and 62nd Avenue North

This intersection is currently unsignalized. Based on unsignalized intersection analysis all movements should operate at an acceptable level of service during AM and PM peak hours with peak season traffic and with peak season plus project traffic, as shown in table 6.

Project Access A and 66th Avenue North

This project access is proposed to have full access to 66th Avenue North. Based on unsignalized intersection analysis all movements should operate at an acceptable level of



TABLE 5
OPTION B
ESTIMATED INTERSECTION LEVEL OF SERVICE
(SIGNALIZED)

Intersection	Time Period	Peak Season		Peak Season Plus	
		Traffic	Project Traffic	Traffic	Project Traffic
113th Street and 66th Avenue north	AM PM	B A		B A	



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TABLE 6
OPTION B
ESTIMATED INTERSECTION LEVEL OF SERVICE
(UNSIGNALIZED)

Intersection	Direction	AM Peak Hour						PM Peak Hour					
		Peak Season			Peak Season Plus			Peak Season			Peak Season Plus		
		Left Delay	Through Delay	Right Delay	Left Delay	Through Delay	Right Delay	Left Delay	Through Delay	Right Delay	Left Delay	Through Delay	Right Delay
1116th Street and 66th Avenue North	EB	9.1	9.1	-	10.4	10.4	-	7.7	7.7	-	8.8	8.8	-
	WB	-	8.2	8.2	-	8.8	8.8	-	8.3	8.3	-	11.1	11.1
	SB	12.1	-	12.1	13.5	-	13.5	8.4	-	8.4	9.7	-	9.7
113th Street and 62nd Avenue	EB	30.0	30.0	30.0	63.5	63.5	63.5	24.5	24.5	24.5	31.5	31.5	31.5
	WB	12.1	12.1	12.1	12.2	12.2	12.2	-	-	-	-	-	-
	NB	18.2	*	*	20.0	*	*	13.8	*	*	16.2	*	*
Project Access A and 66th Avenue North	SB	12.5	*	*	12.6	*	*	16.1	*	*	16.6	*	*
	WB	-	-	-	7.4	7.4	-	-	-	-	7.5	7.5	-
	NB	-	-	-	9.0	-	9.0	-	-	-	8.8	-	8.8
Project Access B and Evergreen Avenue	EB	-	-	-	7.9	7.9	-	-	-	-	7.6	7.6	-
	WB	-	-	-	-	7.4	7.4	-	-	-	-	8.0	8.0
	SB	-	-	-	7.7	-	7.7	-	-	-	7.7	-	7.7

* Free flow therefore delay not calculated.



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service during AM and PM peak hours with peak season plus project traffic, as shown in table 6.

Project Access B and Evergreen Avenue

This project access is proposed to have full access to Evergreen Avenue and align with Irving Avenue. Based on unsignalized intersection analysis all movements should operate within acceptable level of service during AM and PM peak hours with peak season plus project traffic, as shown in Table 6.

ACCESS RECOMMENDATIONS

The recommendations included in this report are based on a field review of the site, the proposed site plan and the Transportation Analysis. The methodology utilized to determine the need for a left and/or right turn lane was based on NCHRP Report 279. The access recommendations for Options A and B are summarized in Tables 7 and 8 respectively and described in the following paragraphs:

Option A

113th Street and 66th Avenue North

As shown in Table 7, the existing northbound left turn lane is approximately 140 feet. The length to accommodate the peak season plus project traffic would be 360 feet. It should be noted the existing northbound left turn lane does not accommodate the peak season traffic.



TABLE 7
ACCESS RECOMMENDATIONS
OPTION A

<u>Intersection</u>	<u>Movement</u>	<u>Volume (1)</u>	<u>Warranted? (2)</u>	<u>Queue Storage (3)</u>	<u>Deceleration Length (4)</u>	<u>Total Length</u>	<u>Existing Length</u>
113th Street and 66th Avenue North	NBL	136/283	Existing	175'	185'	360'	140'
Project Access A and 66th Avenue North	EBR WBL	0/0 50/168	No No	- -	- -	- -	- -

(1) Based on Figure 6 of the report

(2) Based on NCHRP Report #279

(3) Based on Sim Traffic

113th Street and 66th Avenue North

NBL - 162'
Use 175'

(4) Based on 45 MPH design speed on 113th Street (posted speed plus 5 MPH).



Project Access A and 66th Avenue North

This project access is proposed to have full access to 66th Avenue North. Based on the projected volumes an eastbound right turn lane and a westbound left turn lane are not warranted, as shown in table 7.

Option B

113th Street and 66th Avenue North

As shown in Table 8, the existing northbound left turn lane is approximately 140 feet. The length to accommodate the peak season plus project traffic would be 335+ feet. It should be noted the existing northbound left turn lane does not accommodate the peak season traffic.

113th Street and 62nd Avenue

As shown in Table 8, the existing northbound left turn lane is approximately 145 feet. The length required to accommodate the peak season plus project traffic is 260 feet. It should be noted the existing northbound left turn lane does not accommodate the peak season traffic.

Project Access A and 66th Avenue North

This project access is proposed to have full access to 66th Avenue North. Based on the projected volumes an eastbound right turn lane and a westbound left turn lane are not warranted, as shown in Table 8.

Project Access B and Evergreen Avenue

This project access is proposed to have full access to Evergreen Avenue and align with Irving Avenue. Based on the projected volumes a southbound right turn lane and a westbound right turn lane are not warranted, as shown in Table 8.



TABLE 8
ACCESS RECOMMENDATIONS
OPTION B

<u>Intersection</u>	<u>Movement</u>	<u>Volume (1)</u>	<u>Warranted? (2)</u>	<u>Queue Storage (3)</u>	<u>Deceleration Length (4)</u>	<u>Total Length</u>	<u>Existing Length</u>
113th Street and 66th Avenue North	NBL	115/216	Existing	150'	185'	335'	140'
113th Street and 62nd Avenue	NBL	23/84	Existing	75'	185'	260'	145'
Project Access A and 66th Avenue North	EBR WBL	0/0 23/76	No No	- -	- -	- -	- -
Project Access B and Evergreen Avenue	SBR WBR	0/0 1/8	No No	- -	- -	- -	- -

(1) Based on Figure 7 of the report

(2) Based on NCHRP Report #279

(3) Queue Storage Calculations

113th Street and 66th Avenue North

NBL - 149' Use 150' Based on Sim Traffic

113th Street and 62nd Avenue

NBL - 84/30 x 25' = 70' Use 75'

(4) Based on 45 MPH design speed on 113th Street (posted speed plus 5 MPH).



APPENDIX



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PRELIMINARY SITE PLAN



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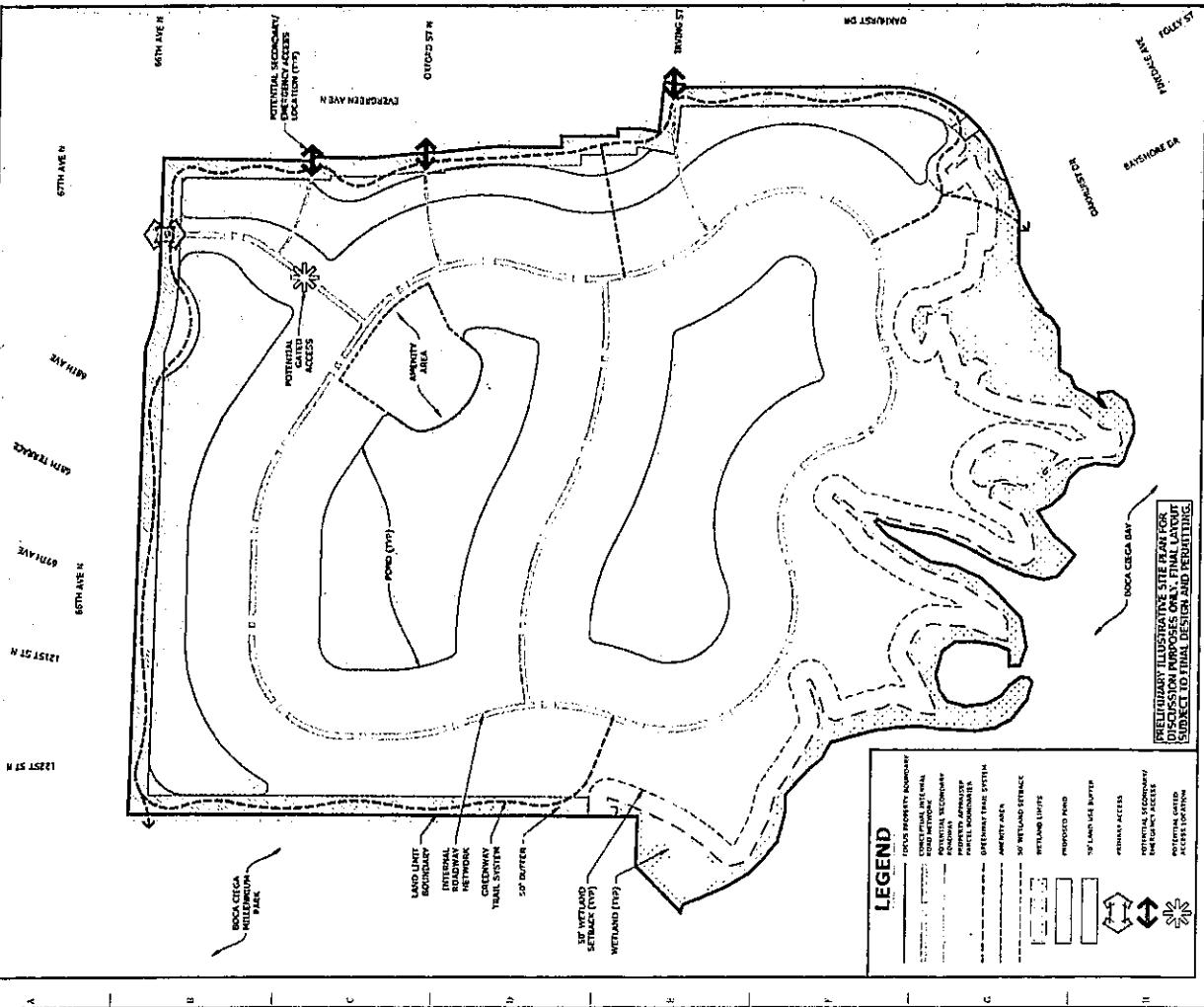
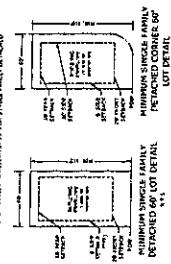
ARDURRA

PINELLAS COUNTY, FLORIDA
RESTORATION BAY
DEVELOPMENT MASTER PLAN

1041

SITE DATA	
NAME OF PROPERTY	1000 N. BROAD ST., PHILADELPHIA, PA.
TYPE OF PROPERTY	OFFICE BUILDING
YEAR BUILT	1910
EXISTING UTILITIES	WATER, SEWER, ELECTRIC, TELEPHONE, GAS
PROPOSED USES	
1. <input type="checkbox"/> OFFICE	<input type="checkbox"/> INDUSTRIAL
2. <input type="checkbox"/> RESIDENTIAL	<input type="checkbox"/> RETAIL
3. <input type="checkbox"/> COMMERCIAL	<input type="checkbox"/> OTHER
EXPLANATION	
I HEREBY CERTIFY THAT THE INFORMATION CONTAINED IN THIS FORM IS TRUE AND CORRECT.	
Dated: 10/10/01 By: JOHN D. HARRIS John D. Harris, Inc.	

TYPICAL LITERATURE



TRIP GENERATION



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PERIOD SETTING

Analysis Name : Weekday
Project Name : Restoration Bay-Golf Coarse **No :**
Date: 11/11/2019 **City:**
State/Province: **Zip/Postal Code:**
Country: **Client Name:**
Analyst's Name: **Edition:** Trip Generation Manual, 10th Ed
(0) indicates small sample size, use carefully.

Land Use	Independent Variable	Size	Time Period	Method	Entry	Exit	Total
430 - Golf Course (General Urban/Suburban)	Holes	18	Weekday	Average 30.38	274(0) 50%	273(0) 50%	547(0)

TRAFFIC REDUCTIONS

Land Use	Entry Reduction	Adjusted Entry	Exit Reduction	Adjusted Exit
430 - Golf Course	0 %	274	0 %	273

EXTERNAL TRIPS

Land Use	External Trips	Pass-by%	Pass-by Trips	Non-pass-by Trips
430 - Golf Course	547	0	0	547

ITE DEVIATION DETAILS

Weekday

Landuse No deviations from ITE.

Methods No deviations from ITE.

External Trips 430 - Golf Course (General Urban/Suburban)
 ITE does not recommend a particular pass-by% for this case.

SUMMARY

Total Entering	274
Total Exiting	273
Total Entering Reduction	0
Total Exiting Reduction	0
Total Entering Internal Capture Reduction	0
Total Exiting Internal Capture Reduction	0
Total Entering Pass-by Reduction	0
Total Exiting Pass-by Reduction	0
Total Entering Non-Pass-by Trips	274
Total Exiting Non-Pass-by Trips	273

PERIOD SETTING

Land Use	Independent Variable	Size	Time Period	Method	Entry	Exit	Total
430 - Golf Course (General Urban/Suburban)	Holes	18	Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.	Average 1.76	25 78%	7 22%	32

TRAFFIC REDUCTIONS

Land Use	Entry Reduction	Adjusted Entry	Exit Reduction	Adjusted Exit
430 - Golf Course	0 %	25	0 %	7

EXTERNAL TRIPS

Land Use	External Trips	Pass-by%	Pass-by Trips	Non-pass-by Trips
430 - Golf Course	32	0	0	32

ITE DEVIATION DETAILS

Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Landuse No deviations from ITE

Methods No deviations from ITE

External Trips 430 - Golf Course (General Urban/Suburban)
ITE does not recommend a particular pass-by% for this case.

SUMMARY

Total Entering	25
Total Exiting	7
Total Entering Reduction	0
Total Exiting Reduction	0
Total Entering Internal Capture Reduction	0
Total Exiting Internal Capture Reduction	0
Total Entering Pass-by Reduction	0
Total Exiting Pass-by Reduction	0
Total Entering Non-Pass-by Trips	25
Total Exiting Non-Pass-by Trips	7

PERIOD SETTING

Analysis Name : Weekday, Peak Hour of
Adjacent Street Traffic, One
Hour Between 4 and 6 p.m.

Project Name : Restoration Bay-Golf Coarse **No :**

Date: 11/11/2019 **City:**

State/Province: **Zip/Postal Code:**

Country: **Client Name:**

Analyst's Name: **Edition:** Trip Generation Manual, 10th
Ed

Land Use	Independent Variable	Size	Time Period	Method	Entry	Exit	Total
430 - Golf Course (General Urban/Suburban)	Holes	18	Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.	Average 2.91	28 54%	24 46%	52

TRAFFIC REDUCTIONS

Land Use	Entry Reduction	Adjusted Entry	Exit Reduction	Adjusted Exit
430 - Golf Course	0 %	28	0 %	24

EXTERNAL TRIPS

Land Use	External Trips	Pass-by%	Pass-by Trips	Non-pass-by Trips
430 - Golf Course	52	0	0	52

ITE DEVIATION DETAILS**Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.**

Landuse No deviations from ITE.

Methods No deviations from ITE.

External Trips 430 - Golf Course (General Urban/Suburban)
ITE does not recommend a particular pass-by% for this case.

SUMMARY

Total Entering	28
Total Exiting	24
Total Entering Reduction	0
Total Exiting Reduction	0
Total Entering Internal Capture Reduction	0
Total Exiting Internal Capture Reduction	0
Total Entering Pass-by Reduction	0
Total Exiting Pass-by Reduction	0
Total Entering Non-Pass-by Trips	28
Total Exiting Non-Pass-by Trips	24

PERIOD SETTING

Analysis Name : Weekday, Peak Hour of
 Adjacent Street Traffic, One
 Hour Between 7 and 9 a.m.
Project Name : RESTORATION BAY **No :**
Date: 10/15/2019 **City:**
State/Province: **Zip/Postal Code:**
Country: **Client Name:**
Analyst's Name: **Edition:** Trip Generation Manual, 10th
 Ed

Land Use	Independent Variable	Size	Time Period	Method	Entry	Exit	Total
210 - Single-Family Detached Housing (General Urban/Suburban)	Dwelling Units	273	Weekday	Best Fit (LOG) $\ln(T) = 0.92\ln(X) + 2.71$	1310 50%	1309 50%	2619

TRAFFIC REDUCTIONS

Land Use	Entry Reduction	Adjusted Entry	Exit Reduction	Adjusted Exit
210 - Single-Family Detached Housing	0 %	1310	0 %	1309

EXTERNAL TRIPS

Land Use	External Trips	Pass-by%	Pass-by Trips	Non-pass-by Trips
210 - Single-Family Detached Housing	2619	0	0	2619

ITE DEVIATION DETAILS

Weekday

Landuse No deviations from ITE.

Methods No deviations from ITE.

External Trips 210 - Single-Family Detached Housing (General Urban/Suburban)
 ITE does not recommend a particular pass-by% for this case.

SUMMARY

Total Entering	1310
Total Exiting	1309
Total Entering Reduction	0
Total Exiting Reduction	0
Total Entering Internal Capture Reduction	0
Total Exiting Internal Capture Reduction	0
Total Entering Pass-by Reduction	0
Total Exiting Pass-by Reduction	0
Total Entering Non-Pass-by Trips	1310
Total Exiting Non-Pass-by Trips	1309

PERIOD SETTING

Analysis Name : Weekday, Peak Hour of
 Adjacent Street Traffic, One
 Hour Between 7 and 9 a.m.
Project Name : RESTORATION BAY **No :**
Date: 10/15/2019 **City:**
State/Province: **Zip/Postal Code:**
Country: **Client Name:**
Analyst's Name: **Edition:** Trip Generation Manual, 10th
 Ed

Land Use	Independent Variable	Size	Time Period	Method	Entry	Exit	Total
210 - Single-Family Detached Housing (General Urban/Suburban)	Dwelling Units	273	Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.	Best Fit (LIN) $T = 0.71(X) + 4.8$	50 25%	149 75%	199

TRAFFIC REDUCTIONS

Land Use	Entry Reduction	Adjusted Entry	Exit Reduction	Adjusted Exit
210 - Single-Family Detached Housing	0 %	50	0 %	149

EXTERNAL TRIPS

Land Use	External Trips	Pass-by%	Pass-by Trips	Non-pass-by Trips
210 - Single-Family Detached Housing	199	0	0	199

ITE DEVIATION DETAILS

Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 7 and 9 a.m.

Landuse No deviations from ITE.

Methods No deviations from ITE.

External Trips 210 - Single-Family Detached Housing (General Urban/Suburban)
 ITE does not recommend a particular pass-by% for this case.

SUMMARY

Total Entering	50
Total Exiting	149
Total Entering Reduction	0
Total Exiting Reduction	0
Total Entering Internal Capture Reduction	0
Total Exiting Internal Capture Reduction	0
Total Entering Pass-by Reduction	0
Total Exiting Pass-by Reduction	0
Total Entering Non-Pass-by Trips	50
Total Exiting Non-Pass-by Trips	149

PERIOD SETTING

Analysis Name : Weekday, Peak Hour of
Adjacent Street Traffic, One
Hour Between 4 and 6 p.m.

Project Name : RESTORATION BAY **No :**

Date: 10/15/2019 **City:**

State/Province: **Zip/Postal Code:**

Country: **Client Name:**

Analyst's Name: **Edition:** Trip Generation Manual, 10th
Ed

Land Use	Independent Variable	Size	Time Period	Method	Entry	Exit	Total
210 - Single-Family Detached Housing (General Urban/Suburban)	Dwelling Units	273	Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.	Best Fit (LOG) $Ln(T) = 0.96Ln(X) + 0.2$	168	98	266

TRAFFIC REDUCTIONS

Land Use	Entry Reduction	Adjusted Entry	Exit Reduction	Adjusted Exit
210 - Single-Family Detached Housing	0 %	168	0 %	98

EXTERNAL TRIPS

Land Use	External Trips	Pass-by%	Pass-by Trips	Non-pass-by Trips
210 - Single-Family Detached Housing	266	0	0	266

ITE DEVIATION DETAILS

Weekday, Peak Hour of Adjacent Street Traffic, One Hour Between 4 and 6 p.m.

Landuse No deviations from ITE.

Methods No deviations from ITE.

External Trips 210 - Single-Family Detached Housing (General Urban/Suburban)
ITE does not recommend a particular pass-by% for this case.

SUMMARY

Total Entering	168
Total Exiting	98
Total Entering Reduction	0
Total Exiting Reduction	0
Total Entering Internal Capture Reduction	0
Total Exiting Internal Capture Reduction	0
Total Entering Pass-by Reduction	0
Total Exiting Pass-by Reduction	0
Total Entering Non-Pass-by Trips	168
Total Exiting Non-Pass-by Trips	98

PINELLAS COUNTY CIP



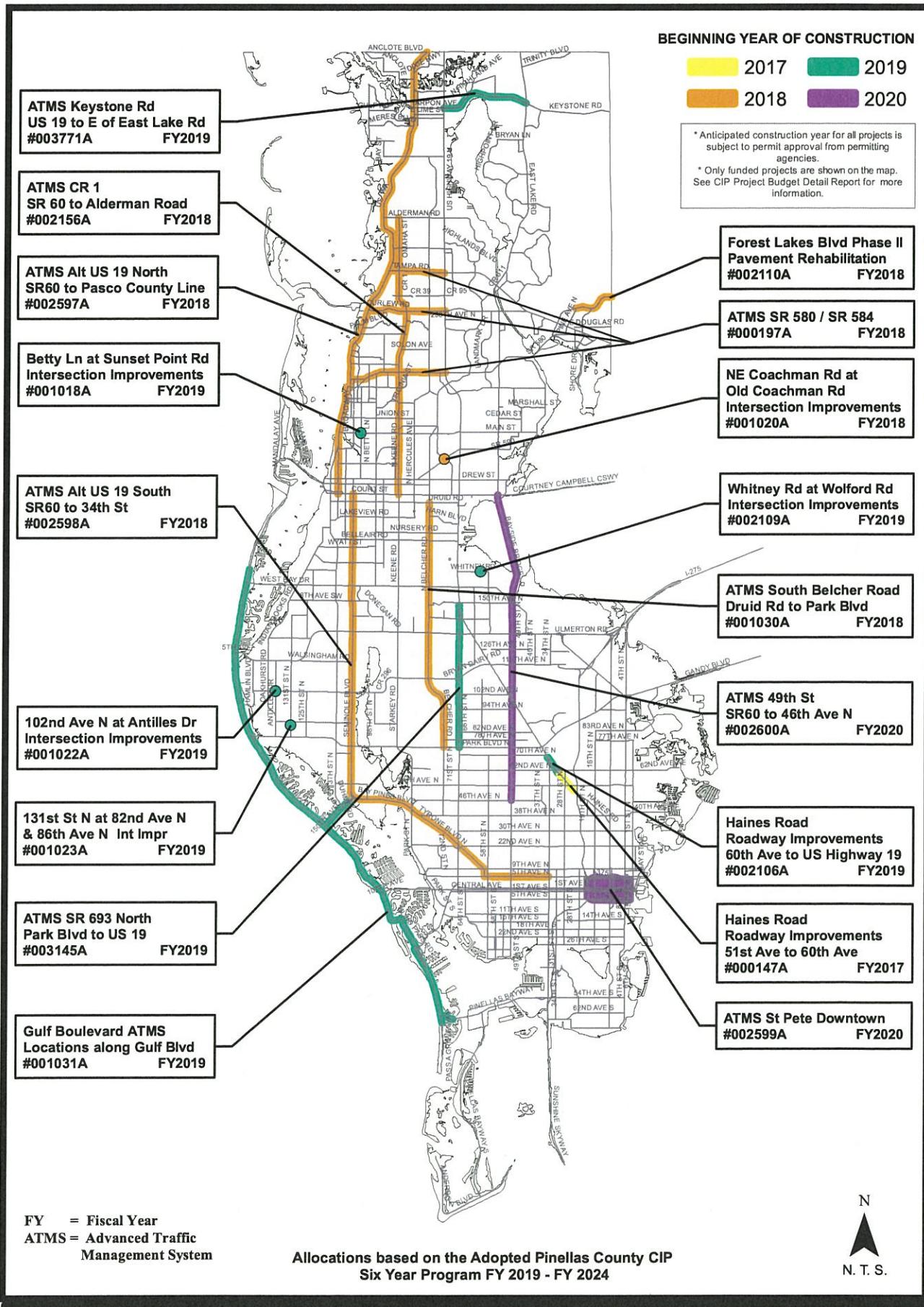
LINCKS & ASSOCIATES, INC.



Capital Improvement Program

Six-Year Work Plan:
FY19 - FY24

www.pinellascounty.org/budget



**PINELLAS COUNTY WORK PROGRAM
FY 2019 through FY 2024
Major Transportation and ATMS Projects**

In preparing this material, every effort has been made to ensure that the information provided is correct. The information is provided as a public service and Pinellas County assumes no liability for any inaccuracies that it may contain.

TRAFFIC COUNTS



LINCKS & ASSOCIATES, INC.



N/S Street: 113th St

N1

Speed: 40 MPH

National Data & Surveying Services

Site Code: 19-3596-003

Date: 9/17/2019

Weather: Sunny

City: Seminole

County: Pinellas

Count Times: 07:00 – 09:00
16:00 - 18:00

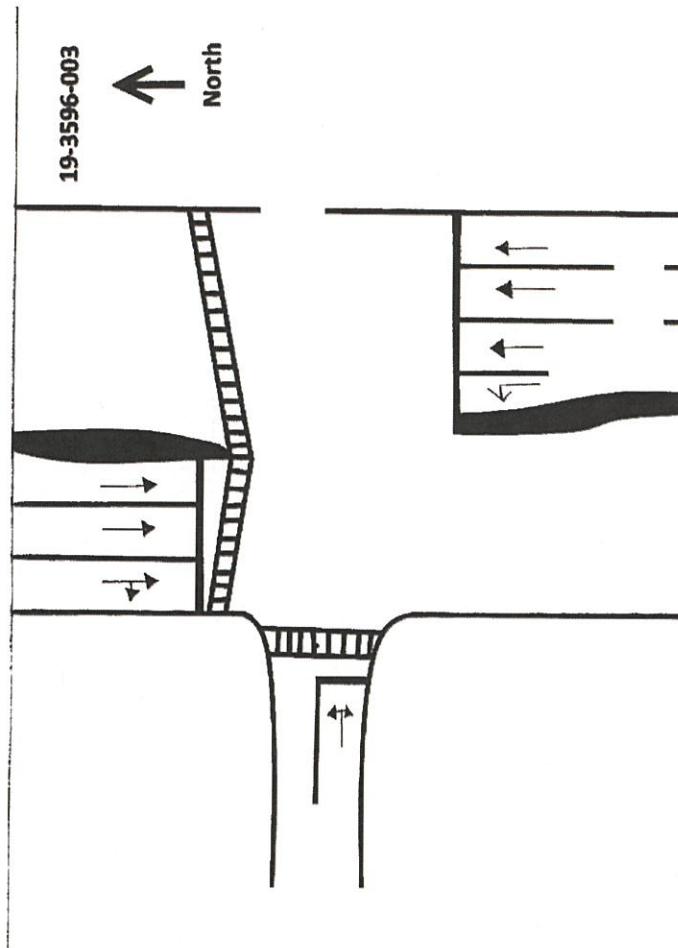
Control: Signalized

SIGNAL TIMING

PHASES	1	2	3
NL/NT	-	00:11	00:18
NT/ST	01:23	01:04	01:24
EL	00:31	00:23	00:17

E/W Street: 66th Ave N

Speed: 30 MPH

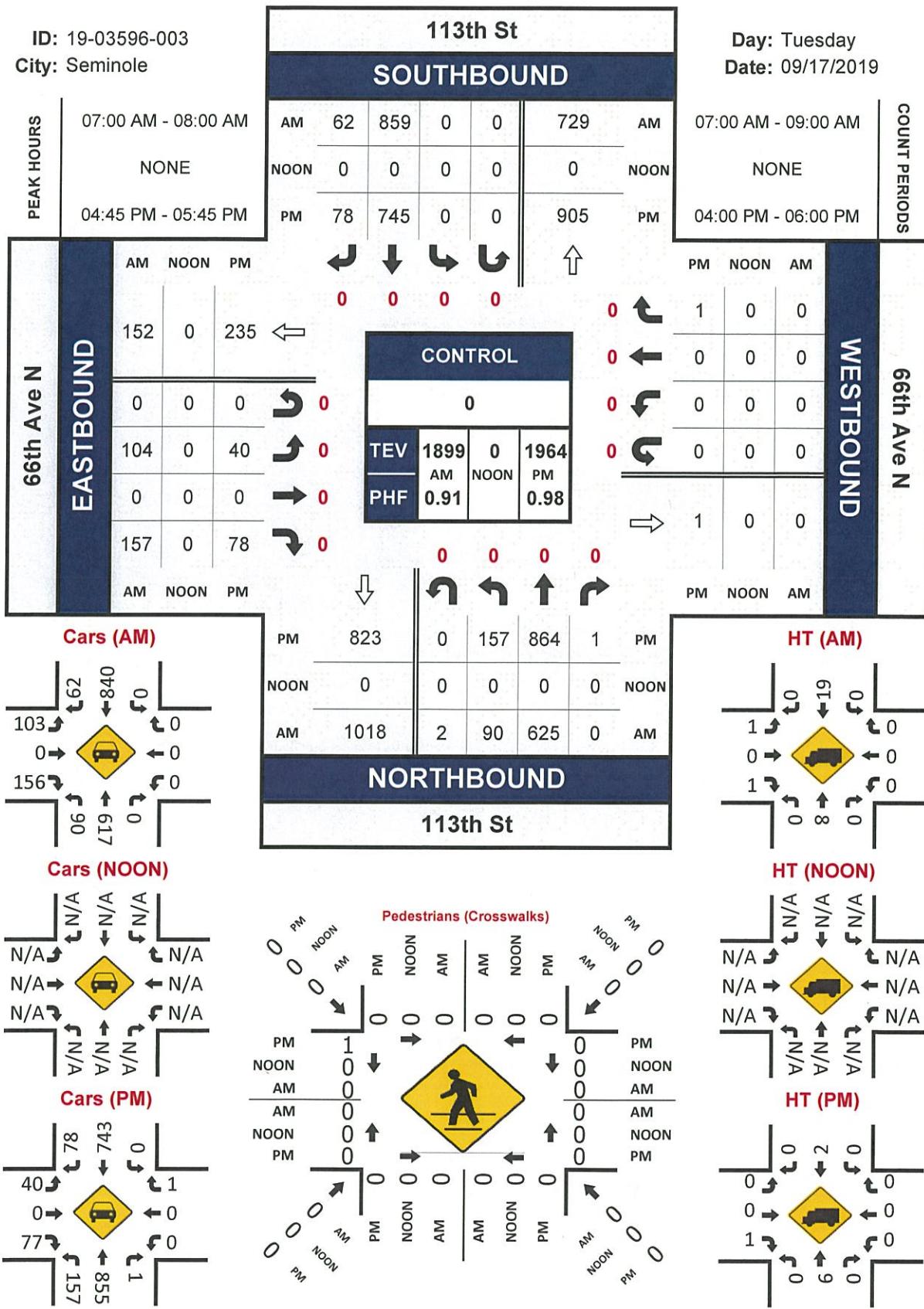


113th St & 66th Ave N

Peak Hour Turning Movement Count

ID: 19-03596-003
City: Seminole

Day: Tuesday
Date: 09/17/2019



National Data & Surveying Services

Location: 113th St & 66th Ave N
 City: Seminole
 Control:

Intersection Turning Movement Count

Project ID: 19-03596-003
 Date: 9/17/2019

Total

NS/EW Streets:		113th St				113th St				66th Ave N				66th Ave N			
		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WR		WL		WR	
AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:00 AM	14	123	0	1	0	227	9	0	19	0	33	0	0	0	0	0	
7:15 AM	23	157	0	1	0	242	9	0	16	0	41	0	0	0	0	0	
7:30 AM	33	174	0	0	0	202	31	0	37	0	45	0	0	0	0	0	
7:45 AM	20	171	0	0	0	188	13	0	32	0	38	0	0	0	0	0	
8:00 AM	18	133	0	0	0	190	5	0	25	0	34	0	0	0	0	0	
8:15 AM	22	147	0	0	0	172	11	0	23	0	18	0	0	0	0	0	
8:30 AM	10	146	0	0	0	178	8	0	15	0	35	0	0	0	0	0	
8:45 AM	8	109	0	0	0	165	9	0	17	0	26	0	0	0	0	0	
TOTAL VOLUMES:	148	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
APPROACH %'s:	11.30%	88.55%	0.00%	0.15%	0.00%	94.27%	5.73%	0.00%	40.53%	0.00%	59.47%	0.00%	0	0	0	0	
PEAK HR:	07:00 AM - 08:00 AM				0	2	0	859	62	0	104	0	157	0	0	0	
PEAK HR VOL:	90	625	0	0.500	0.000	0.887	0.500	0.000	0.703	0.000	0.872	0.000	0.000	0.000	0.000	0.000	
PEAK HR FACTOR:	0.682	0.898	0.000	0.866				0.917			0.796						
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:00 PM	25	202	1	0	0	153	9	0	15	0	12	0	0	0	0	0	
4:15 PM	23	181	0	0	0	184	16	0	15	0	15	0	0	0	0	0	
4:30 PM	48	218	0	0	0	154	20	0	14	0	19	0	0	0	0	0	
4:45 PM	37	218	0	0	0	191	24	0	15	0	17	0	0	0	0	0	
5:00 PM	47	227	0	0	0	173	15	0	9	0	18	0	0	0	0	0	
5:15 PM	37	224	1	0	0	169	21	0	8	0	25	0	0	1	0	0	
5:30 PM	36	195	0	0	0	212	18	0	8	0	18	0	0	0	0	0	
5:45 PM	28	185	1	0	0	186	14	0	9	0	23	0	0	0	0	0	
TOTAL VOLUMES:	281	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
APPROACH %'s:	14.52%	85.27%	0.16%	0.05%	0.00%	91.21%	8.79%	0.00%	38.75%	0.00%	61.25%	0.00%	0.00%	0.00%	100.00%	0.00%	
PEAK HR:	04:45 PM - 05:45 PM				0	0	745	78	0	40	0	78	0	0	0	0	
PEAK HR VOL:	157	864	1	0	0.000	0.879	0.813	0.000	0.667	0.000	0.780	0.000	0.000	0.000	0.250	0.000	
PEAK HR FACTOR:	0.835	0.952	0.250	0.932			0.895			0.894							

National Data & Surveying Services

Intersection Turning Movement Count

Location: 113th St & 66th Ave N
 City: Seminole
 Control: 0

Project ID: 19-03596-003
 Date: 9/17/2019

Cars

NS/EW Streets:		113th St				113th St				66th Ave N				66th Ave N			
		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WR		WU	
AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
7:00 AM	14	122	0	1	0	227	9	0	19	0	32	0	0	0	0	TOTAL 424	
7:15 AM	23	154	0	1	0	241	9	0	16	0	41	0	0	0	0	485	
7:30 AM	33	172	0	0	0	194	31	0	37	0	45	0	0	0	0	512	
7:45 AM	20	169	0	0	0	178	13	0	31	0	38	0	0	0	0	449	
8:00 AM	17	130	0	0	0	189	5	0	25	0	34	0	0	0	0	400	
8:15 AM	21	145	0	0	0	170	11	0	23	0	18	0	0	0	0	388	
8:30 AM	10	144	0	0	0	176	8	0	14	0	34	0	0	0	0	386	
8:45 AM	8	108	0	0	0	163	8	0	17	0	25	0	0	0	0	329	
TOTAL VOLUMES:	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	EL	WL	WT	WR	TOTAL 3373
APPROACH %'s:	146	1144	0	2	0	1538	94	0	182	0	267	0	0	0	0	0	
PEAK HR:	11.30%	88.54%	0.00%	0.15%	0.00%	94.24%	5.76%	0.00%	40.53%	0.00%	59.47%	0.00%	0	0	0	0	
PEAK HR VOL:	90	617	0	2	0	840	62	0	103	0	156	0	0	0	0	TOTAL 1870	
PEAK HR FACTOR:	0.68	0.897	0.000	0.500	0.000	0.871	0.500	0.000	0.696	0.000	0.867	0.000	0.000	0.000	0.000	0.913	
		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WR		WU	
PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:00 PM	24	199	1	0	0	153	9	0	13	0	12	0	0	0	0	TOTAL 411	
4:15 PM	23	179	0	0	0	184	16	0	15	0	14	0	0	0	0	431	
4:30 PM	48	217	0	0	0	153	19	0	14	0	18	0	0	0	0	469	
4:45 PM	37	215	0	0	0	191	24	0	15	0	16	0	0	0	0	498	
5:00 PM	47	226	0	0	0	171	15	0	9	0	18	0	0	0	0	486	
5:15 PM	37	219	1	0	0	169	21	0	8	0	25	0	0	1	0	481	
5:30 PM	36	195	0	0	0	212	18	0	8	0	18	0	0	0	0	487	
5:45 PM	28	185	1	0	0	185	14	0	9	0	23	0	0	0	0	446	
TOTAL VOLUMES:	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	EL	WL	WT	WR	TOTAL 3709
APPROACH %'s:	280	1635	3	1	0	1418	136	0	91	0	144	0	0	0	1	0	
PEAK HR:	14.59%	85.20%	0.16%	0.05%	0.00%	91.25%	8.75%	0.00%	38.72%	0.00%	61.28%	0.00%	0.00%	0.00%	100.00%	0.00%	
PEAK HR VOL:	157	855	1	0	0	743	78	0	40	0	77	0	0.000	0.770	0.250	TOTAL 1952	
PEAK HR FACTOR:	0.84	0.946	0.250	0.000	0.000	0.876	0.813	0.000	0.667	0.000	0.886	0.000	0.000	0.250	0.000	0.980	

National Data & Surveying Services

Intersection Turning Movement Count

Location: 113th St & 66th Ave N
 City: Seminole
 Control: 0

Project ID: 19-03596-003
 Date: 9/17/2019

Bikes

NS/EW Streets:		113th St				113th St				66th Ave N				66th Ave N			
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL 0	
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
7:30 AM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	1	
7:45 AM	0	2	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
TOTAL VOLUMES:	0	NT 3	NR 0	NU 0	SL 0	ST 0	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	TOTAL 6	
APPROACH %'s:	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	TOTAL 6	
PEAK HR:	07:00 AM - 08:00 AM																
PEAK HR VOL:	0	3	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL 4	
PEAK HR FACTOR:	0.000	0.375	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.500	
		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL 1	
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:00 PM	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	2	
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	
TOTAL VOLUMES:	0	NT 2	NR 0	NU 0	SL 0	ST 5	SR 0	SU 0	EL 0	ET 0	ER 0	EU 0	WL 0	WT 0	WR 0	TOTAL 7	
APPROACH %'s:	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	100.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	0.00%	TOTAL 7	
PEAK HR:	04:45 PM - 05:45 PM																
PEAK HR VOL:	0	1	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL 4	
PEAK HR FACTOR:	0.00	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.375	0.000	0.000	0.000	0.000	0.500	

National Data & Surveying Services

Intersection Turning Movement Count

Location: 113th St & 66th Ave N

City: Seminole

Project ID: 19-03596-003
Date: 9/17/2019

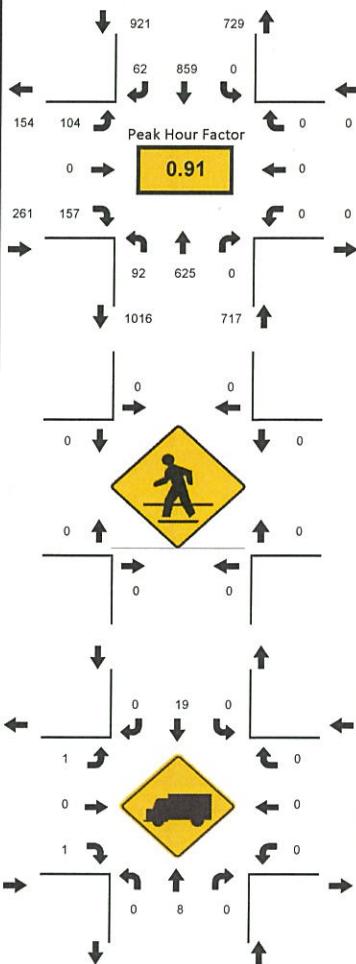
Pedestrians (Crosswalks)

NS/EW Streets:	113th St		113th St		66th Ave N		66th Ave N		TOTAL
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
AM	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0
8:00 AM	0	3	0	0	0	0	0	0	5
8:15 AM	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0
8:45 AM	1	0	0	0	0	0	0	0	1
TOTAL VOLUMES :	1	WB	EB	WB	NB	SB	NB	SB	TOTAL
APPROACH %'s :	25.00%	75.00%			0	0	0.00%	100.00%	6
PEAK Hr :	07:00 AM - 08:00 AM		0	0	0	0	0	0	TOTAL 0
PEAK HR VOL :	0								
PEAK HR FACTOR :									

PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL
	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	0	WB	EB	WB	NB	SB	NB	SB	TOTAL 1
APPROACH %'s :	0	0	0	0	0	0	0.00%	100.00%	
PEAK Hr :	04:45 PM - 05:45 PM		0	0	0	0	0	0	
PEAK HR VOL :	0								
PEAK HR FACTOR :									

LOCATION: 113th St & 66th Ave N
CITY/STATE: Seminole, FL

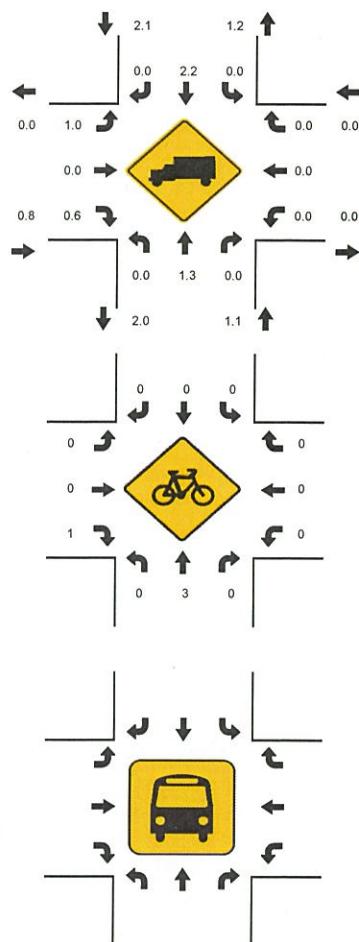
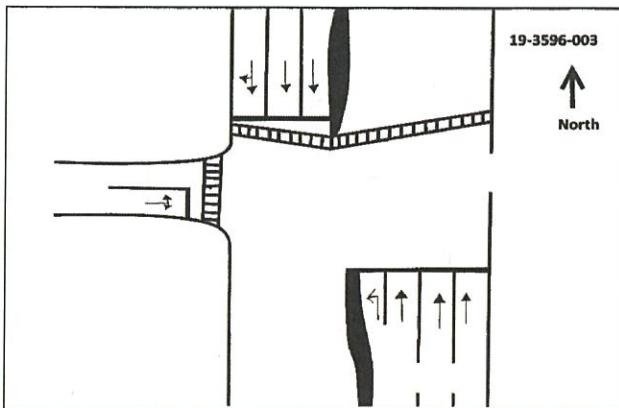
PROJECT ID: 19-03596-003
DATE: 09/17/2019



Peak-Hour: 07:00 AM - 08:00 AM
Peak 15-Minute: 07:30 AM - 07:45 AM



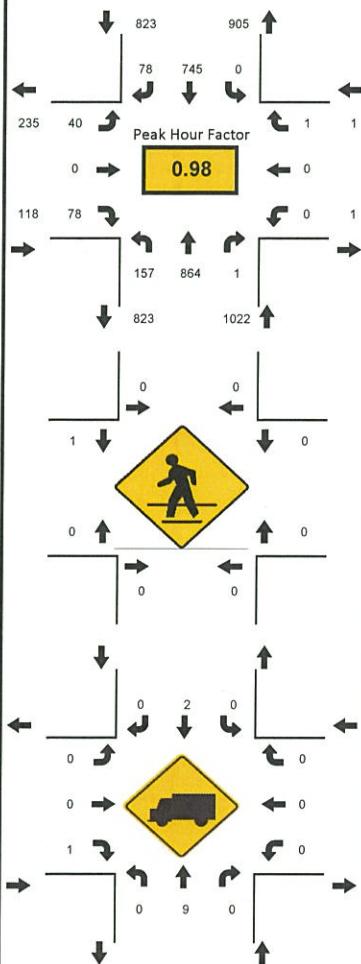
National Data & Surveying Services



15-Min Count Period Beginning At	113th St Northbound					113th St Southbound					66th Ave N Eastbound					66th Ave N Westbound					Total	Hourly Total	
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*			
07:00 AM	14	123	0	1		0	227	9	0		19	0	33	0		0	0	0	0		426	1899	
07:15 AM	23	157	0	1		0	242	9	0		16	0	41	0		0	0	0	0		489	1878	
07:30 AM	33	174	0	0		0	202	31	0		37	0	45	0		0	0	0	0		522	1782	
07:45 AM	20	171	0	0		0	188	13	0		32	0	38	0		0	0	0	0		462	1652	
08:00 AM	18	133	0	0		0	190	5	0		25	0	34	0		0	0	0	0		405	1524	
08:15 AM	22	147	0	0		0	172	11	0		23	0	18	0		0	0	0	0		393	1119	
08:30 AM	10	146	0	0		0	178	8	0		15	0	35	0		0	0	0	0		392	726	
08:45 AM	8	109	0	0		0	165	9	0		17	0	26	0		0	0	0	0		334	334	
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound					Total		
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*			
All Vehicles	132	696	0	4		0	968	124	0		148	0	180	0		0	0	0	0		2252		
Heavy Trucks	0	12	0			0	40	0			4	0	4			0	0	0			60		
Pedestrians		0					0						0				0					0	
Bicycles		0					0					0					0					12	
Railroad		0					0					0					0						
Stopped Buses		0					0					0					0						

LOCATION: 113th St & 66th Ave N
CITY/STATE: Seminole, FL

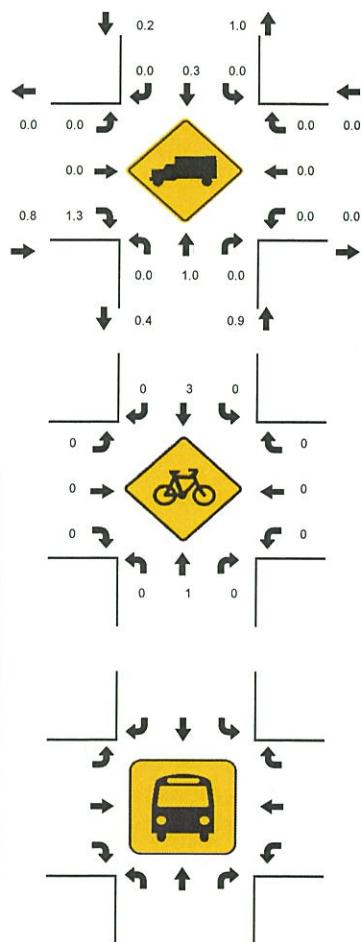
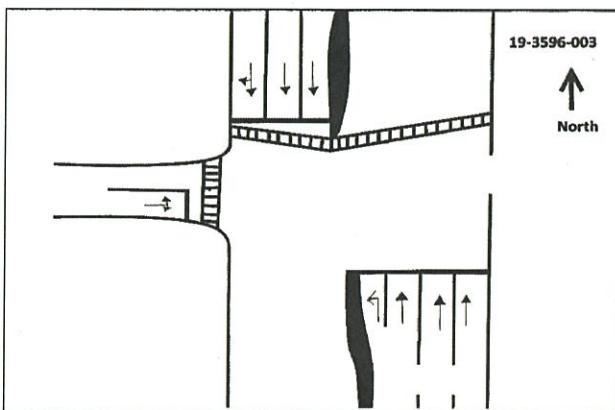
PROJECT ID: 19-03596-003
DATE: 09/17/2019



Peak-Hour: 04:45 PM - 05:45 PM
Peak 15-Minute: 04:45 PM - 05:00 PM



National Data & Surveying Services



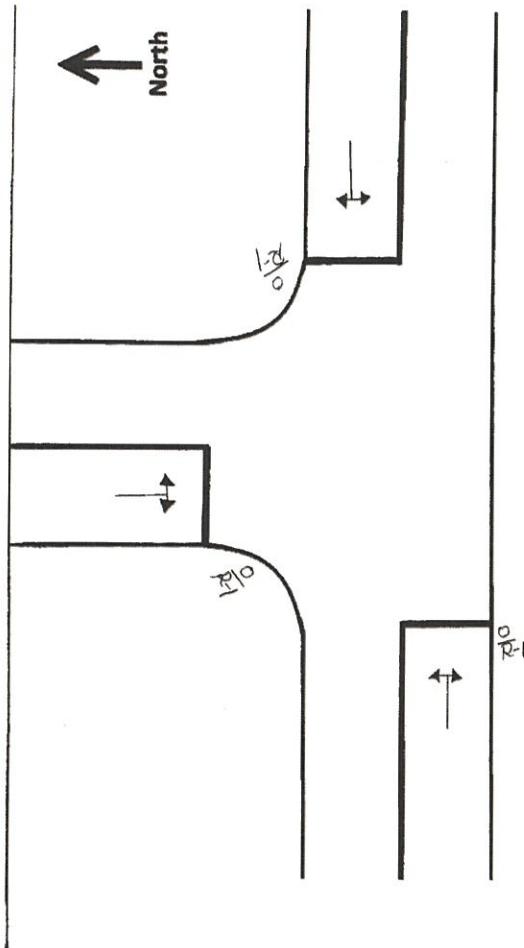


National Data & Surveying Services

N/S Street: 116th St N

Speed: 30 MPH

E/W Street: 66th Ave N	Speed: 30 MPH
------------------------	---------------



N1

Site Code: 19-3596-002
Date: 9/17/2019
Weather: Sunny
City: Seminole
County: Pinellas
Count Times: 07:00 – 09:00
16:00 - 18:00
Control: 3-Way Stop (SB/EB/WB)

116th St N & 66th Ave N

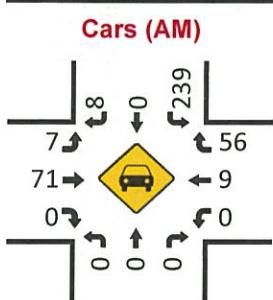
Peak Hour Turning Movement Count

ID: 19-03596-002
City: Seminole

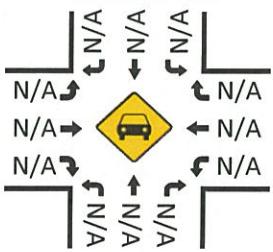
		116th St N							
		SOUTHBOUND							
PEAK HOURS	07:15 AM - 08:15 AM	AM	8	0	239	0	64	AM	07:00 AM - 09:00 AM
	NONE	NOON	0	0	0	0	0	NOON	NONE
	04:30 PM - 05:30 PM	PM	15	0	79	0	153	PM	04:00 PM - 06:00 PM
	AM NOON PM	◀	▼	◀	◀	↑	PM NOON AM		
	17 0 86	◀	0	0	0	0	145 0 57		
	0 0 0	◀	0				71 0 9		
	7 0 8	◀	0				0 0 0		
	71 0 28	▶	0				0 0 0		
	0 0 0	◀	0				107 0 310		
	AM NOON PM	↓	◀	◀	↑	◀	PM NOON AM		
		CONTROL							
		0							
		TEV	391	0	346				
		PHF	0.75	AM	NOON	PM	0.98		
		0	0	0	0	0	0		

66th Ave N EASTBOUND

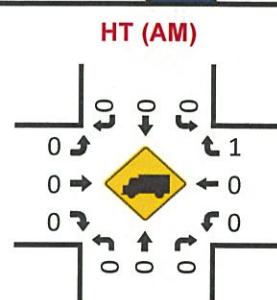
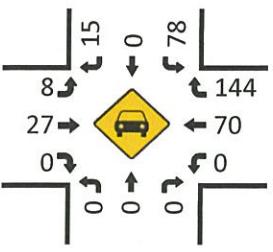
WESTBOUND 66th Ave N



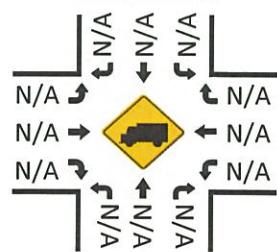
Cars (NOON)



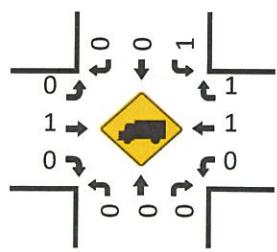
Cars (PM)



HT (NOON)



HT (PM)



National Data & Surveying Services

Location: 116th St N & 66th Ave N

City: Seminole

Control

Intersection Turning Movement Count

Project ID: 19-03596-002
Date: 9/17/2019

NS/EW Streets:		116th St N				116th St N				66th Ave N				66th Ave N					
AM		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND			
NL	NT	NR	NU	SL	ST	SU	EL	ER	EU	WL	WT	WR	WL	WT	WR	WL	WT	WR	
7:00 AM	0	0	0	0	24	0	5	18	0	0	0	0	1	18	0	0	0	0	
7:15 AM	0	0	0	0	34	0	0	17	0	0	0	0	0	12	0	0	0	0	
7:30 AM	0	0	0	0	88	0	2	19	0	0	0	0	1	9	0	0	0	0	
7:45 AM	0	0	0	0	89	0	2	0	3	18	0	0	0	3	15	0	0	0	
8:00 AM	0	0	0	0	28	0	2	0	17	0	0	0	0	5	21	0	0	0	
8:15 AM	0	0	0	0	21	0	0	0	18	0	0	0	0	6	19	0	0	0	
8:30 AM	0	0	0	0	35	0	0	0	3	12	0	0	0	4	15	0	0	0	
8:45 AM	0	0	0	0	21	0	0	2	12	0	0	0	0	2	16	0	0	0	
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	0	0	NR	NU	SL	ST	SU	EL	ER	EU	WL	WT	WR	WL	WT	WR	WL	WT	
PEAK HR VOL : PEAK HR FACTOR :	0	0	0	0	97.70%	0.00%	2.30%	0.00%	11.49%	88.51%	0.00%	0.00%	0.00%	14.97%	85.03%	0.00%	14.97%	85.03%	0.00%
0.000	0.000	0.000	0.000	0.671	0.000	1.000	0.000	0.438	0.934	0	0	0	0.450	0.679	0.000	0.635	0.635	0.752	
PM		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND			
NL	NT	NR	NU	SL	ST	SU	EL	ER	EU	WL	WT	WR	WL	WT	WR	WL	WT	WR	
4:00 PM	0	0	0	0	15	0	2	1	8	0	0	0	0	6	25	0	0	0	
4:15 PM	0	0	0	0	11	0	2	0	2	7	0	0	0	0	13	18	0	0	
4:30 PM	0	0	0	0	15	0	2	0	2	6	0	0	0	0	22	36	0	0	
4:45 PM	0	0	0	0	16	0	4	0	1	8	0	0	0	0	17	42	0	0	
5:00 PM	0	0	0	0	19	0	4	0	3	8	0	0	0	0	19	34	0	0	
5:15 PM	0	0	0	0	29	0	5	0	2	6	0	0	0	0	13	33	0	0	
5:30 PM	0	0	0	0	24	0	5	0	1	5	0	0	0	0	15	33	0	0	
5:45 PM	0	0	0	0	17	0	1	0	0	12	0	0	0	0	16	22	0	0	
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	0	0	NR	NU	SL	ST	SU	EL	ER	EU	WL	WT	WR	WL	WT	WR	WL	WT	
PEAK HR VOL : PEAK HR FACTOR :	0	0	0	0	85.38%	0.00%	14.62%	0.00%	16.67%	83.33%	0.00%	0.00%	0.00%	33.24%	66.76%	0.00%	33.24%	66.76%	0.00%
0.000	0.000	0.000	0.000	0.681	0.000	0.750	0.000	0.667	0.875	0.000	0.000	0.000	0.807	0.863	0.000	0.915	0.915	0.983	

National Data & Surveying Services

Intersection Turning Movement Count

Location: 116th St N & 66th Ave N
 City: Seminole
 Control: 0

Project ID: 19-03596-002
 Date: 9/17/2019

Cars

NS/EW Streets:	116th St N				116th St N				66th Ave N				66th Ave N			
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WL		WT	
	AM	NL	NT	NR	SL	ST	SU	EU	EL	ET	ER	EU	EL	ST	SR	0
7:00 AM	0	0	0	0	24	0	0	5	18	0	0	0	1	17	0	0
7:15 AM	0	0	0	0	34	0	2	0	17	0	0	0	0	12	0	0
7:30 AM	0	0	0	0	88	0	2	4	19	0	0	0	1	9	0	0
7:45 AM	0	0	0	0	89	0	2	3	18	0	0	0	3	15	0	0
8:00 AM	0	0	0	0	28	0	2	0	17	0	0	0	0	5	20	0
8:15 AM	0	0	0	0	21	0	0	0	18	0	0	0	0	6	19	0
8:30 AM	0	0	0	0	34	0	0	0	12	0	0	0	0	4	14	0
8:45 AM	0	0	0	0	21	0	0	2	12	0	0	0	0	2	16	0
TOTAL VOLUMES : APPROACH %'s :	0	0	0	0	339	0	8	0	17	131	0	0	0	22	122	0
PEAK HR:	07:15 AM - 08:15 AM				97.69%	0.00%	2.31%	0.00%	11.49%	88.51%	0.00%	0.00%	0.00%	15.28%	84.72%	0.00%
PEAK HR VOL :	0	0	0	0	239	0	8	0	7	71	0	0	0	9	56	0
PEAK HR FACTOR :	0.00	0.000	0.000	0.000	0.671	0.000	1.000	0.000	0.438	0.934	0.000	0.000	0.000	0.450	0.700	0.000
					0.679		0.848							0.650		

NS/EW Streets:	116th St N				116th St N				66th Ave N				66th Ave N			
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WL		WT	
	PM	NL	NT	NR	SL	ST	SU	EU	EL	ET	ER	EU	EL	ST	SR	0
4:00 PM	0	0	0	0	15	0	2	0	1	6	0	0	0	0	0	0
4:15 PM	0	0	0	0	10	0	2	0	2	7	0	0	0	13	18	0
4:30 PM	0	0	0	0	14	0	2	0	2	6	0	0	0	21	36	0
4:45 PM	0	0	0	0	16	0	4	0	1	7	0	0	0	17	41	0
5:00 PM	0	0	0	0	19	0	4	0	3	8	0	0	0	19	34	0
5:15 PM	0	0	0	0	29	0	5	0	2	6	0	0	0	13	33	0
5:30 PM	0	0	0	0	24	0	5	0	1	5	0	0	0	15	33	0
5:45 PM	0	0	0	0	17	0	1	0	0	12	0	0	0	16	22	0
TOTAL VOLUMES : APPROACH %'s :	0	0	0	0	144	0	25	0	12	57	0	0	0	0	241	0
PEAK HR:	04:30 PM - 05:30 PM				85.21%	0.00%	14.79%	0.00%	17.39%	82.61%	0.00%	0.00%	0.00%	33.24%	66.76%	0.00%
PEAK HR VOL :	0	0	0	0	78	0	15	0	8	27	0	0	0	70	144	0
PEAK HR FACTOR :	0.00	0.000	0.000	0.000	0.672	0.000	0.750	0.000	0.667	0.844	0.000	0.000	0.000	0.833	0.878	0.000
					0.684		0.795							0.922		

National Data & Surveying Services

Intersection Turning Movement Count

Location: 116th St N & 66th Ave N
 City: Seminole
 Control: 0

Project ID: 19-03596-002
 Date: 9/17/2019

HT

NS/EW Streets:		116th St N				116th St N				66th Ave N				66th Ave N			
		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WR		WL		WR	
AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
APPROACH %'s :					100.00%	0.00%	0.00%			0	0	0.00%	0.00%	0	0	0.00%	0.00%
PEAK HR:	07:15 AM - 08:15 AM																
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.000
TOTAL VOLUMES :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
APPROACH %'s :																	
PEAK HR:	04:30 PM - 05:30 PM																
PEAK HR VOL :	0	0	0	0	0	1	0	0	0	0	0	0.250	0.000	0.000	0.250	0.000	0.500
PEAK HR FACTOR :	0.00	0.000	0.000	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.000

National Data & Surveying Services

Intersection Turning Movement Count

Location: 116th St N & 66th Ave N
City: Seminole
Control: 0

NS/EW Streets:		116th St N				116th St N				66th Ave N				66th Ave N			
AM		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WR		WL		WR	
NL	NT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0	1
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	1	0	0	0	0	0	0	0	1	0	0	0	1
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	2
8:45 AM	0	0	0	0	2	0	0	0	1	1	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	0	0	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WL	TOTAL
PEAK HR :	07:15 AM - 08:15 AM				100.00%	0.00%	0.00%	0.00%	1	2	0	0	0.00%	100.00%	0.00%	0.00%	0.00%
PEAK HR VOL :	0	0	0	0	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	TOTAL
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	9
PM		NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
NL	NT	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	TOTAL
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	1	0	0	0	0	4	0	0	5
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	1	2
TOTAL VOLUMES : APPROACH %'s :	0	0	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WL	TOTAL
PEAK HR :	04:30 PM - 05:30 PM				100.00%	0.00%	0.00%	0.00%	1	0	0	0	0.00%	100.00%	0.00%	0.00%	0.00%
PEAK HR VOL :	0	0	0	0	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.250	0.000	0.000	5
PEAK HR FACTOR :	0.00	0.00	0.00	0.00	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.250	0.000	0.000	250

National Data & Surveying Services

Intersection Turning Movement Count

Location: 116th St N & 66th Ave N
City: Seminole

Project ID: 19-03596-002

Date: 9/17/2019

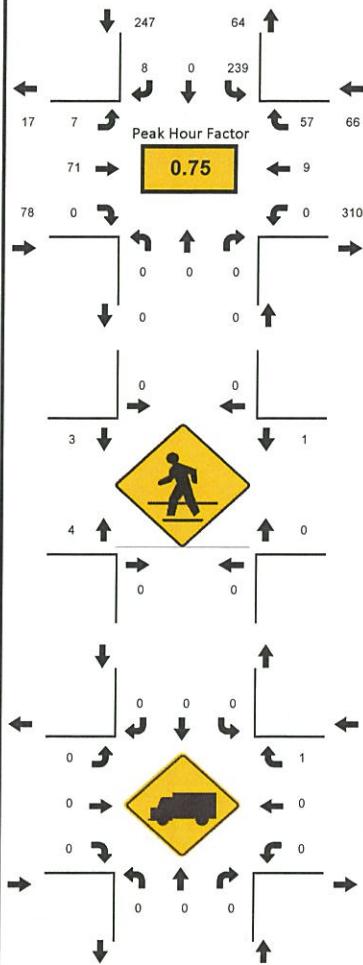
Pedestrians (Crosswalks)

AM	NS/EW Streets:		116th St N		116th St N		66th Ave N		66th Ave N		TOTAL
			NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		
	EB	WB	EB	WB	NB	SB	NB	SB	NB	SB	
7:00 AM	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	2	0	2
7:30 AM	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	1	0	1
8:00 AM	0	0	0	0	0	0	1	1	0	0	1
8:15 AM	1	0	0	0	0	0	0	0	0	0	5
8:30 AM	0	0	0	0	0	0	0	0	0	0	2
8:45 AM	0	0	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES :	1	0	WB	EB	WB	NB	SB	NB	SB	TOTAL	11
APPROACH %'s :	100.00%	0.00%				0.00%	100.00%	50.00%	50.00%		
PEAK HR :	07:15 AM - 08:15 AM									TOTAL	8
PEAK HR VOL :	0	0				0	1	4	3		
PEAK HR FACTOR :						0.250	0.250	0.500	0.250		0.438

PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL	
	EB	WB	EB	WB	NB	SB	NB	SB		
4:00 PM	0	0	0	0	1	0	0	0	1	
4:15 PM	2	1	0	0	0	2	0	0	5	
4:30 PM	0	0	0	0	0	0	1	0	1	
4:45 PM	0	0	0	0	2	0	0	0	2	
5:00 PM	0	0	0	0	0	3	0	0	3	
5:15 PM	0	0	0	0	0	0	0	0	0	
5:30 PM	0	0	0	0	0	0	0	0	0	
5:45 PM	0	0	0	0	0	0	0	0	0	
TOTAL VOLUMES :	2	1	WB	EB	WB	NB	SB	NB	TOTAL	12
APPROACH %'s :	66.67%	33.33%			37.50%	62.50%	100.00%	0.00%		
PEAK HR :	04:30 PM - 05:30 PM								TOTAL	6
PEAK HR VOL :	0	0			0	2	3	1		
PEAK HR FACTOR :					0.250	0.250	0.417	0.250		0.500

LOCATION: 116th St N & 66th Ave N
CITY/STATE: Seminole, FL

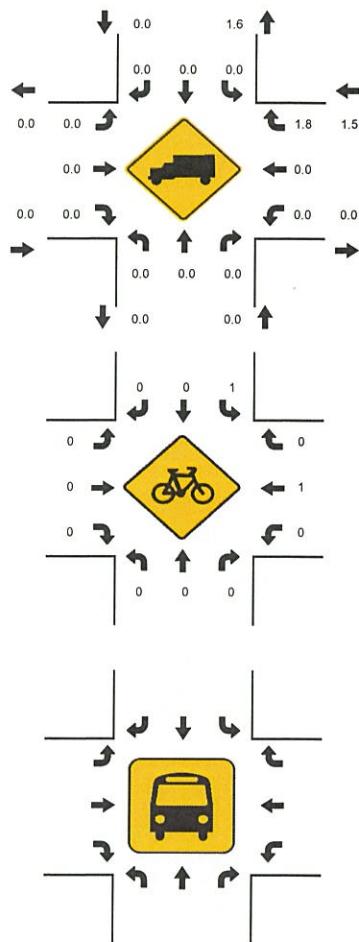
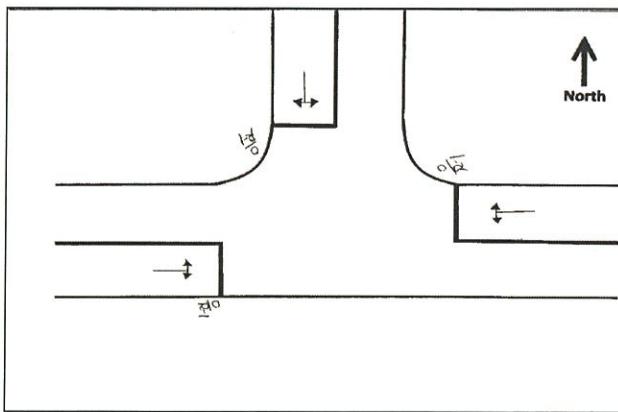
PROJECT ID: 19-03596-002
DATE: 09/17/2019



Peak-Hour: 07:15 AM - 08:15 AM
Peak 15-Minute: 07:45 AM - 08:00 AM

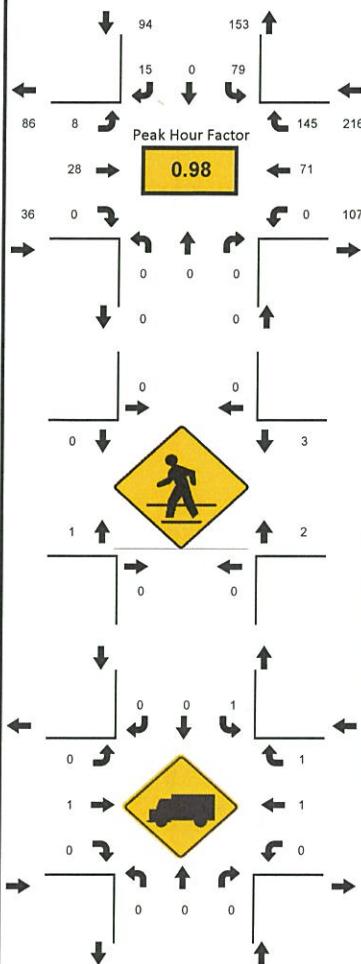


National Data & Surveying Services



LOCATION: 116th St N & 66th Ave N
CITY/STATE: Seminole, FL

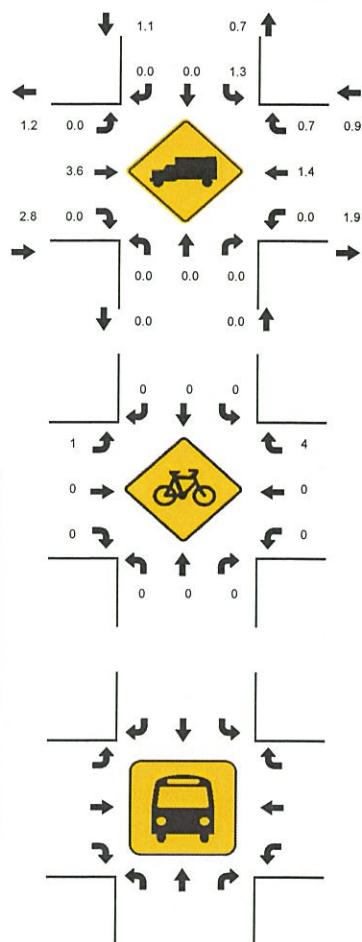
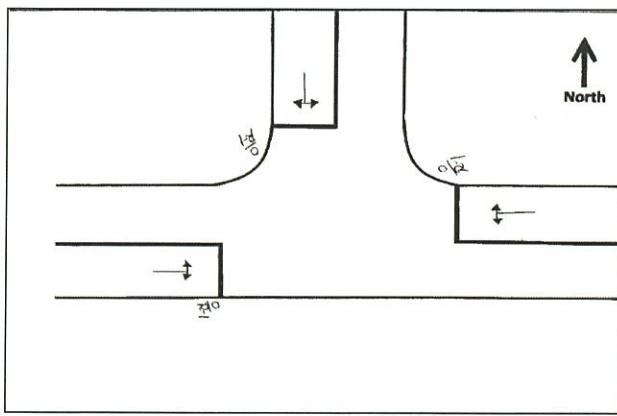
PROJECT ID: 19-03596-002
DATE: 09/17/2019



Peak-Hour: 04:30 PM - 05:30 PM
Peak 15-Minute: 04:45 PM - 05:00 PM



National Data & Surveying Services





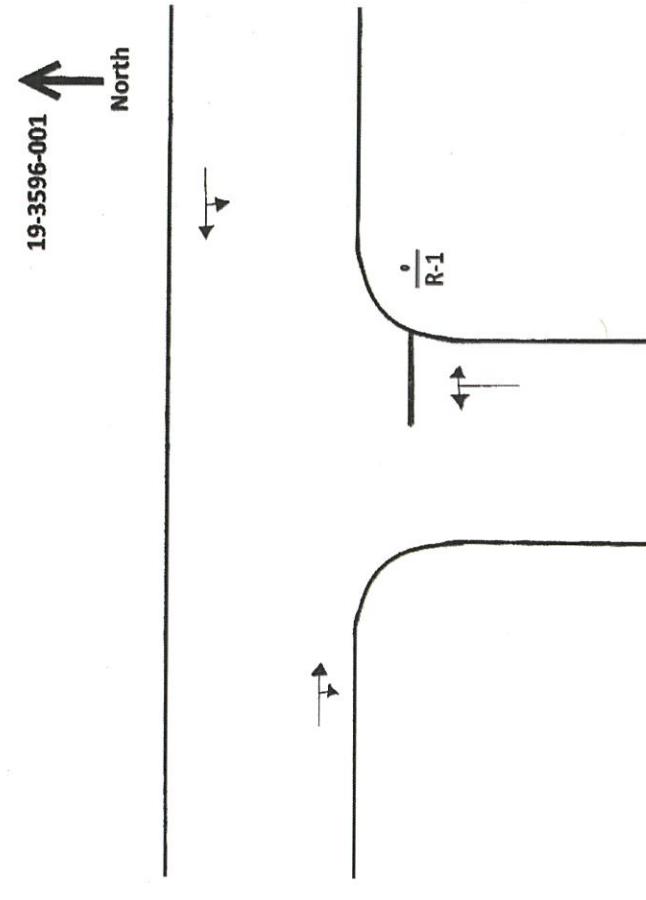
National Data & Surveying Services



N/S Street: Evergreen Ave N

Speed: 25 MPH

E/W Street: 66th Ave N	Speed: 30 MPH
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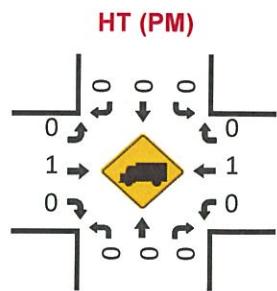
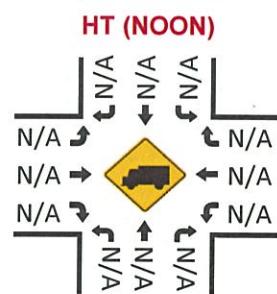
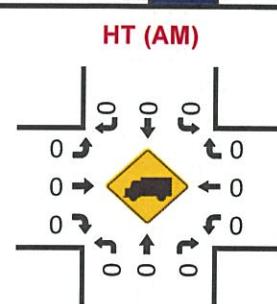
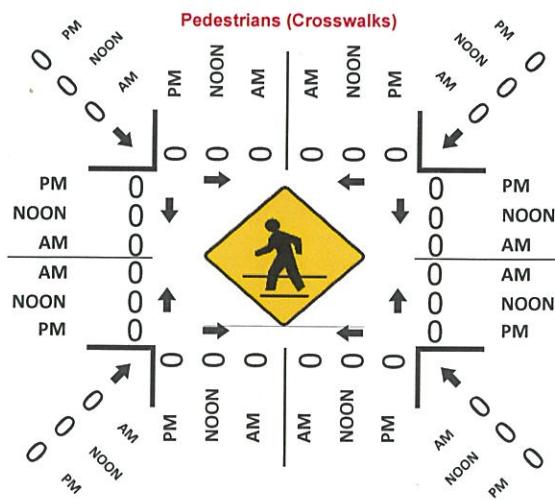
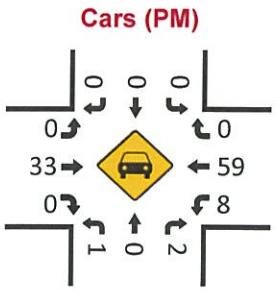
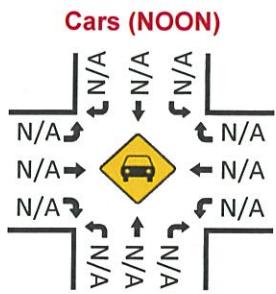
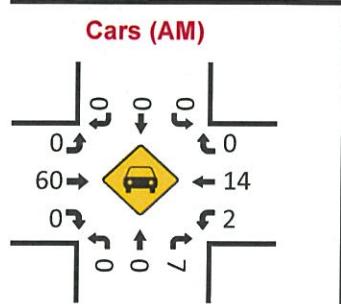


Site Code: 19-3596-001
Date: 9/17/2019
Weather: Sunny
City: Seminole
County: Pinellas
Count Times: 07:00 – 09:00
16:00 - 18:00
Control: 1-Way Stop (NB)

Evergreen Ave N & 66th Ave N

Peak Hour Turning Movement Count

ID: 19-03596-001



National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & 66th Ave N
City: Seminole
Control:

NS/EW Streets:		Evergreen Ave N				Evergreen Ave N				66th Ave N				66th Ave N			
AM		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND	
NL	NT	NR	NU	SL	SR	EL	ER	WL	WT	EU	WR	WL	WT	EU	WR	WL	WT
7:00 AM	0	0	2	0	0	0	0	16	1	0	0	0	0	0	0	0	0
7:15 AM	0	0	2	0	0	0	0	12	0	0	0	1	0	0	0	0	0
7:30 AM	0	0	3	0	0	0	0	15	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	19	0	0	0	4	0	0	0	0	0
8:00 AM	0	0	2	0	0	0	0	15	0	0	0	1	6	0	0	0	0
8:15 AM	0	0	2	0	0	0	0	11	0	0	0	1	4	0	0	0	0
8:30 AM	1	0	2	0	0	0	0	12	0	0	0	1	1	0	0	0	0
8:45 AM	1	0	0	0	0	0	0	12	0	0	0	0	1	0	0	0	0
TOTAL VOLUMES : APPROACH %'s :	13.33%	0.00%	13	NU	SL	ST	SR	SU	EL	ET	ER	WL	WT	EU	WR	WL	WT
PEAK HR:	07:30 AM - 08:30 AM				0	0	0	0	0	0	112	1	0	0	0	0	0
PEAK HR VOL :	0	0	7	0	0	0	0	0	0.00%	99.12%	0.88%	0.00%	19.05%	80.95%	0.00%	0.00%	0.00%
PEAK HR FACTOR :	0.000	0.000	0.583	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.789	0.000	0.500	0.583	0.000	0.000	0.000
TOTAL VOLUMES : APPROACH %'s :	33.33%	0.00%	66.67%	0.00%	0	0	0	0	0	0	59	0	0	103	0	0	0
PEAK HR:	04:15 PM - 05:15 PM				0	0	0	0	0.00%	100.00%	0.00%	0.00%	8.85%	91.15%	0.00%	0.00%	0.00%
PEAK HR VOL :	1	0	2	0	0	0	0	0	0.000	0.000	0	0	34	0	8	60	0
PEAK HR FACTOR :	0.250	0.000	0.500	0.000	0.000	0.000	0.000	0.000	0.773	0.000	0.667	0.577	0.000	0.607	0.000	0.000	0.000

National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & 66th Ave N

City: Seminole

Control: 0

Project ID: 19-03596-001
Date: 9/17/2019

PEAK HR:

Cars

NS/EW Streets:	Evergreen Ave N				Evergreen Ave N				66th Ave N				66th Ave N			
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WL		WR	
	NT	NR	NU	SL	ST	SR	EL	ER	EU	0	0	0	WL	WT	WL	WR
7:00 AM	0	0	2	0	0	0	0	0	16	1	0	0	0	0	0	0
7:15 AM	0	0	2	0	0	0	0	0	12	0	0	0	1	0	0	0
7:30 AM	0	0	3	0	0	0	0	0	15	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	19	0	0	0	4	0	0	0
8:00 AM	0	0	2	0	0	0	0	0	15	0	0	0	1	6	0	0
8:15 AM	0	0	2	0	0	0	0	0	11	0	0	0	1	4	0	0
8:30 AM	1	0	2	0	0	0	0	0	12	0	0	0	1	1	0	0
8:45 AM	1	0	0	0	0	0	0	0	12	0	0	0	0	1	0	0
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WL	WR
APPROACH %'s :	2	0	13	0	0	0	0	0	0	112	1	0	4	17	0	0
PEAK HR :	07:30 AM - 08:30 AM				0	0	0	0	0.88%	99.12%	0.88%	0.00%	19.05%	80.95%	0.00%	0.00%
PEAK HR VOL :	0	0	7	0	0.000	0.000	0.000	0.000	0.000	60	0	0	2	14	0	0
PEAK HR FACTOR :	0.00	0.000	0.583	0.000	0.583	0.000	0.000	0.000	0.789	0.000	0.000	0.500	0.583	0.000	0.000	0.865

NS/EW Streets:	Evergreen Ave N				Evergreen Ave N				66th Ave N				66th Ave N			
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WL		WR	
	NT	NR	NU	SL	ST	SR	EL	ER	EU	0	0	0	WL	WT	WL	WR
4:00 PM	0	0	0	0	0	0	0	0	4	0	0	0	0	0	0	0
4:15 PM	1	0	1	0	0	0	0	0	7	0	0	1	4	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	8	0	0	2	9	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	7	0	0	3	15	0	0	0
5:00 PM	0	0	1	0	0	0	0	0	11	0	0	1	9	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	6	0	0	2	26	0	0	0
5:30 PM	0	0	2	0	0	0	0	0	5	0	0	1	13	0	0	0
5:45 PM	1	0	0	0	0	0	0	0	8	0	0	0	12	0	0	0
TOTAL VOLUMES :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WL	WR
APPROACH %'s :	2	0	4	0	0	0	0	0	100.00%	100.00%	0.00%	0.00%	8.93%	91.07%	0.00%	0.00%
PEAK HR :	04:15 PM - 05:15 PM				0	0	0	0	0.750	0.000	0.750	0.750	0.598	0.567	0.000	0.000
PEAK HR VOL :	1	0	2	0	0.500	0.000	0.000	0.000	0.000	33	0	8	59	0	0	0
PEAK HR FACTOR :	0.25	0.000	0.375	0.000	0.375	0.000	0.000	0.000	0.750	0.000	0.667	0.667	0.598	0.567	0.000	0.000

National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & 66th Ave N

City: Seminole

Control: 0

Project ID: 19-03596-001
Date: 9/17/2019

NS/EW Streets:		Evergreen Ave N				Evergreen Ave N				66th Ave N				66th Ave N				
AM		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		
		NL	NT	NR	NU	SL	ST	SU	EL	ET	ER	EU	WL	WT	WR	WL	WT	WR
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WL	WT	WR
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	07:30 AM - 08:30 AM		0		0		0		0		0		0		0		0	
	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PM		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WL	WT	WR
	4:00 PM	0	0	0	0	0	0	0	0	0	2	0	0	0	0	0	0	0
	4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	4:45 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	0
	5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s : PEAK HR :	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WL	WT	WR
	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
	04:15 PM - 05:15 PM		0		0		0		0		0		0		0		0	
	0.00	0.00	0.00	0.00	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.250	0.000	0.250	0.000

National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & 66th Ave N

City: Seminole

Control: 0

Project ID: 19-03596-001
Date: 9/17/2019

Bikes

NS/EW Streets:	Evergreen Ave N				Evergreen Ave N				66th Ave N				66th Ave N			
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WT		WR		WU	
AM	NL	NT	NR	NU	SL	ST	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
8:30 AM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
8:45 AM	0	0	0	0	0	0	0	0	1	0	0	0	0	0	0	1
TOTAL VOLUMES : APPROACH %'s :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3
PEAK HR :	07:30 AM - 08:30 AM															
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.000	0.500
PM	NL	NT	NR	NU	SL	ST	SU	EL	ET	ER	EU	WL	WT	WR	WU	TOTAL
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1
TOTAL VOLUMES : APPROACH %'s :	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	2
PEAK HR :	04:15 PM - 05:15 PM															
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	1	0	0	0	0	0	1
PEAK HR FACTOR :	0.00	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.000	0.250

National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & 66th Ave N

City: Seminole

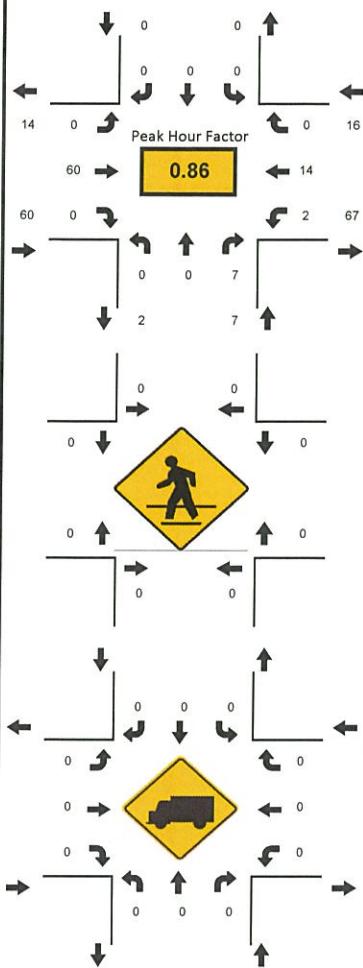
Project ID: 19-03596-001

Date: 9/17/2019

Pedestrians (Crosswalks)

LOCATION: Evergreen Ave N & 66th Ave N
CITY/STATE: Seminole, FL

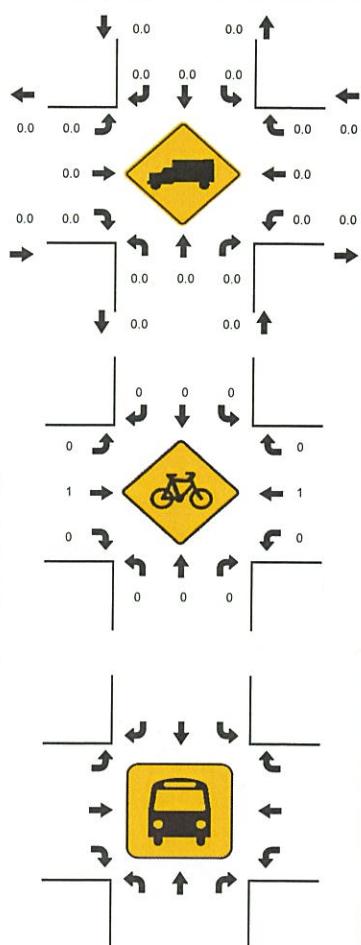
PROJECT ID: 19-03596-001
DATE: 09/17/2019



Peak-Hour: 07:30 AM - 08:30 AM
Peak 15-Minute: 08:00 AM - 08:15 AM

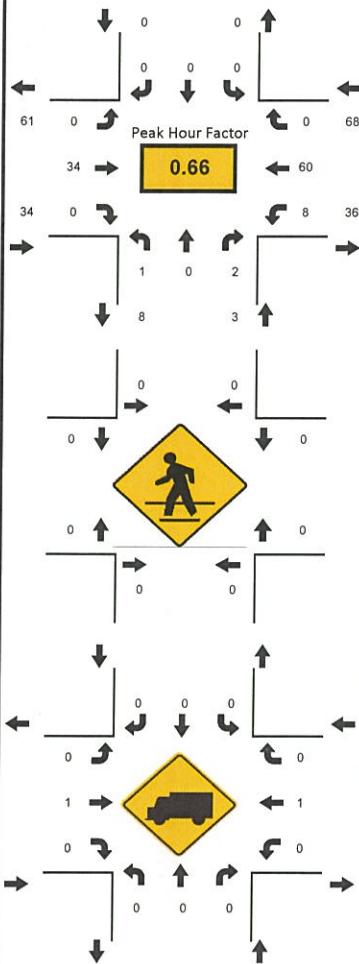


National Data & Surveying Services



LOCATION: Evergreen Ave N & 66th Ave N
CITY/STATE: Seminole, FL

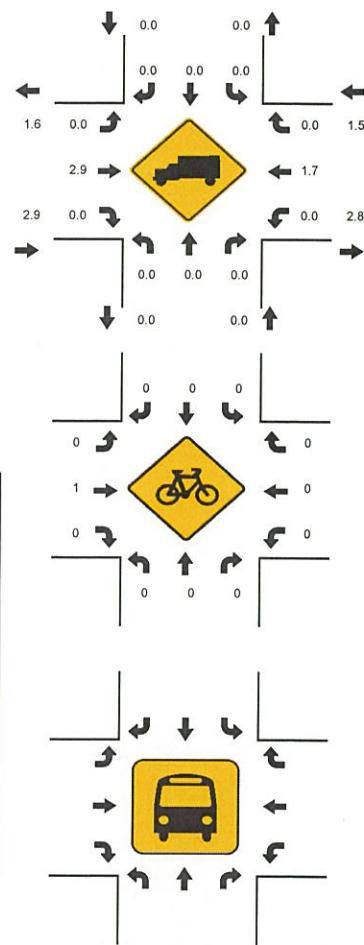
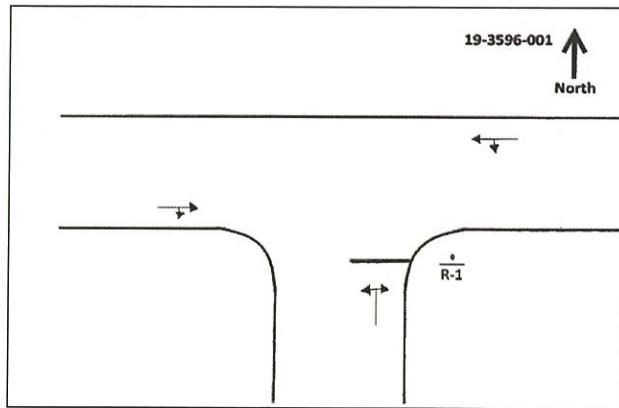
PROJECT ID: 19-03596-001
DATE: 09/17/2019



Peak-Hour: 04:15 PM - 05:15 PM
Peak 15-Minute: 05:00 PM - 05:15 PM



National Data & Surveying Services





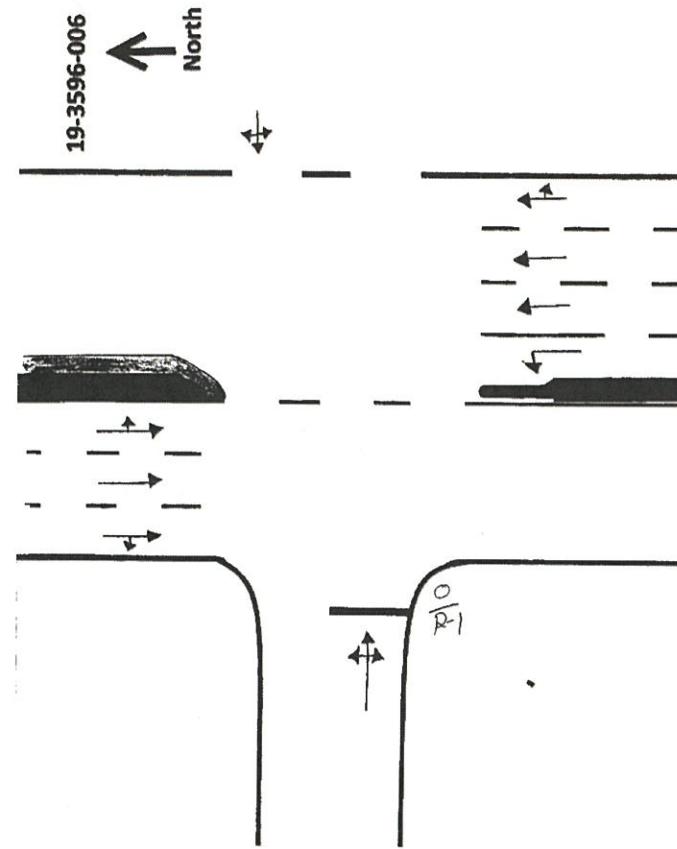
National Data & Surveying Services

N/S Street: 113th St

N↑

Speed: 40 MPH

E/W Street: 62nd Ave	Speed: 25 MPH
----------------------	---------------

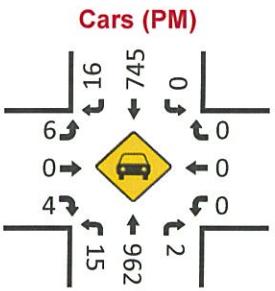
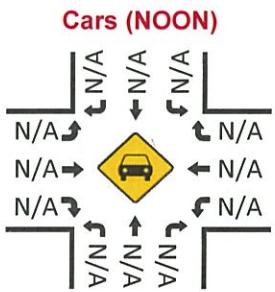
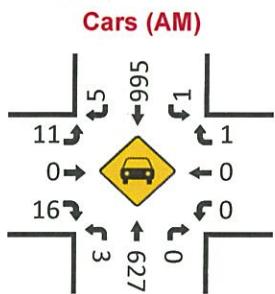


Site Code: 19-3596-006
Date: 9/17/2019
Weather: Sunny
City: Seminole
County: Pinellas
Count Times: 07:00 – 09:00
16:00 - 18:00
Control: 1-Way Stop (EB)

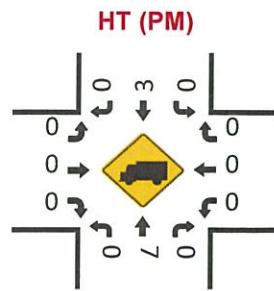
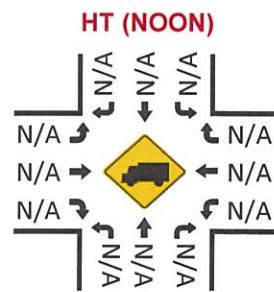
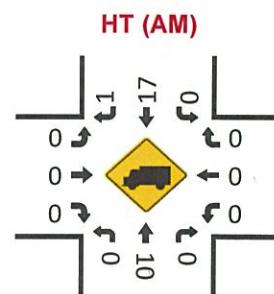
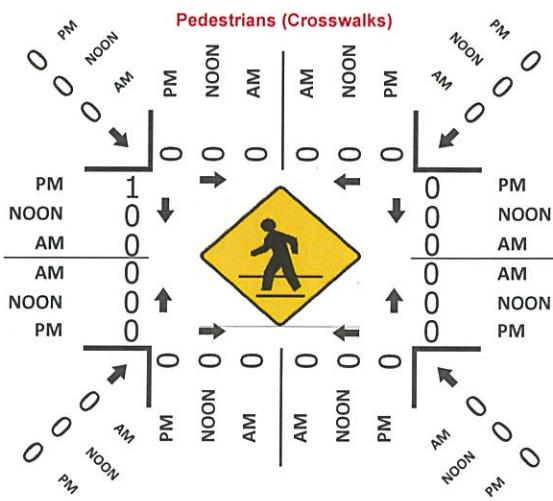
113th St & 62nd Ave

Peak Hour Turning Movement Count

ID: 19-03596-006



PM	752	0	15	969	2	PM
NOON	0	0	0	0	0	NOON
AM	1028	0	3	637	0	AM



National Data & Surveying Services

Location: 113th St & 62nd Ave
City: Seminole
Control:

Intersection Turning Movement Count

City: Seminole Control:

National Data & Surveying Services

Intersection Turning Movement Count

Location: 113th St & 62nd Ave
 City: Seminole
 Control: 0

Project ID: 19-03596-006
 Date: 9/17/2019

Cars

NS/EW Streets:		113th St				113th St				62nd Ave				62nd Ave			
		NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND		EASTBOUND		WESTBOUND	
AM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
7:00 AM	1	110	0	0	0	0	0	0	4	0	4	0	0	0	0	0	
7:15 AM	0	152	0	0	0	0	0	300	1	0	3	0	0	0	1	0	
7:30 AM	1	180	0	0	0	0	0	256	0	1	0	5	0	0	0	0	
7:45 AM	1	159	0	0	0	1	0	206	3	0	4	0	3	0	0	0	
8:00 AM	1	136	0	0	0	0	0	233	1	0	3	0	5	0	0	0	
8:15 AM	0	137	0	0	0	0	0	193	3	0	1	0	0	0	0	0	
8:30 AM	1	133	1	0	0	0	0	205	5	0	3	0	0	0	0	0	
8:45 AM	2	124	1	0	0	0	0	170	2	0	1	0	2	0	0	0	
TOTAL VOLUMES:	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
APPROACH %'s:	7	1131	2	0	1	1774	15	0	20	0	26	0	0	1	0	0	
PEAK HR %:	0.61%	99.21%	0.18%	0.00%	0.05%	99.11%	0.84%	0.00%	43.48%	0.00%	56.52%	0.00%	0.00%	100.00%	0.00%	0.00%	
PEAK HR VOL:	3	627	0	0	1	995	5	0	11	0	16	0	0	1	0	0	
PEAK HR FACTOR:	0.75	0.871	0.000	0.000	0.250	0.829	0.417	0.000	0.688	0.000	0.800	0.000	0.000	0.250	0.000	0.902	
		07:15 AM - 08:15 AM															
PM	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
4:00 PM	5	216	0	0	0	0	0	169	2	0	1	0	0	0	0	0	
4:15 PM	2	194	0	0	0	0	0	182	4	0	1	0	0	0	0	0	
4:30 PM	1	260	0	0	0	0	0	156	4	0	3	0	0	0	0	0	
4:45 PM	3	249	0	0	0	0	0	180	5	1	1	0	0	0	0	0	
5:00 PM	1	241	0	0	0	0	0	179	3	0	2	0	0	0	0	0	
5:15 PM	8	249	0	0	0	0	0	185	5	1	2	0	1	0	0	0	
5:30 PM	3	223	2	0	0	0	0	201	3	0	1	0	0	0	0	0	
5:45 PM	2	200	0	0	0	0	0	198	0	0	2	0	0	0	0	0	
TOTAL VOLUMES:	NL	NT	NR	NU	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU	
APPROACH %'s:	25	1832	2	0	0	0	0	1450	26	2	13	0	5	0	0	0	
PEAK HR %:	1.34%	98.55%	0.11%	0.00%	0.00%	98.11%	1.76%	0.14%	72.22%	0.00%	27.78%	0.00%	0	0	0	0	
PEAK HR VOL:	15	962	2	0	0	745	16	2	6	0	4	0	0	0	0	0	
PEAK HR FACTOR:	0.47	0.966	0.250	0.000	0.000	0.927	0.800	0.500	0.750	0.000	1.000	0.833	0.000	0.000	0.000	0.971	

National Data & Surveying Services

Location: 113th St & 62nd Ave
City: Seminole
Control: 0

Intersection Turning Movement Count

Bikes

NS/EW Streets:	113th St				113th St				62nd Ave				TOTAL	
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WR			
AM	NL	NT	NR	SL	ST	SU	EL	ET	ER	EU	WL	WT	WR	WU
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	3	0	0	0	0	0	0	0	0	0	0	0	1
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	3
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	1
TOTAL VOLUMES: APPROACH %'s:	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK Hr:	07:15 AM - 08:15 AM				0				0				TOTAL 5	
PEAK HR VOL:	0	3	0	0	0	1	0	0	0	0	0	0	0	
PEAK HR FACTOR:	0.000	0.250	0.000	0.000	0.000	0.250	0.000	0.250	0.000	0.000	0.000	0.000	0.000	

National Data & Surveying Services

Intersection Turning Movement Count

Location: 113th St & 62nd Ave

City: Seminole

Project ID: 19-035596-006

Date: 9/17/2019

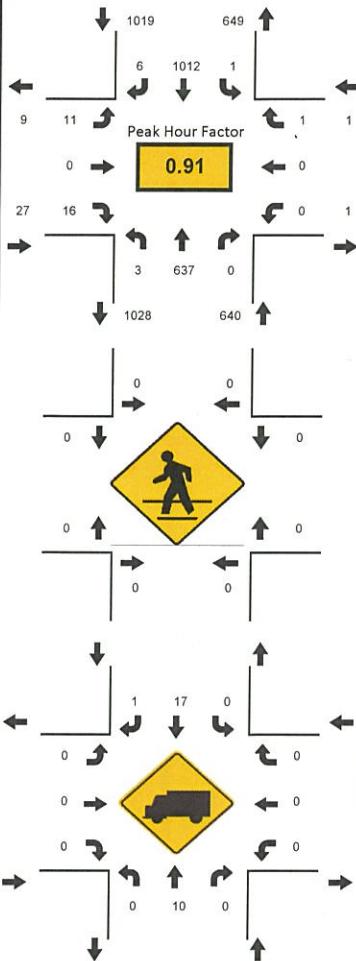
Pedestrians (Crosswalks)

NS/EW Streets:	113th St		113th St		62nd Ave		62nd Ave			
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL	
AM	WB	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
7:00 AM	0	0	0	0	0	1	0	0	0	1
7:15 AM	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	1	0	0	1	2
8:30 AM	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	2	0	0	0	2
TOTAL VOLUMES :	0	0	0	0	0	3	1	0	1	5
APPROACH %'s :					75.00%	25.00%	0.00%	100.00%		
PEAK HR :	07:15 AM - 08:15 AM									
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :										

PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL	
	WB	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
4:00 PM	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	1	1
5:15 PM	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	0	0	0	0	0	0	0	0	1	1
APPROACH %'s :										
PEAK HR :	04:45 PM - 05:45 PM									
PEAK HR VOL :	0	0	0	0	0	0	0	0	1	1
PEAK HR FACTOR :										

LOCATION: 113th St & 62nd Ave
CITY/STATE: Seminole, FL

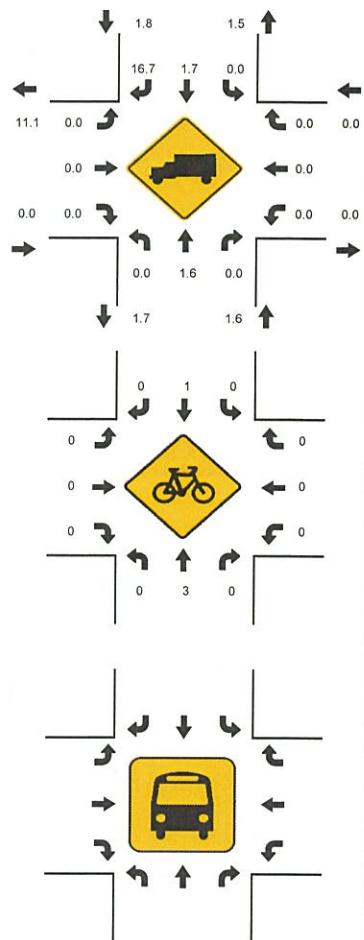
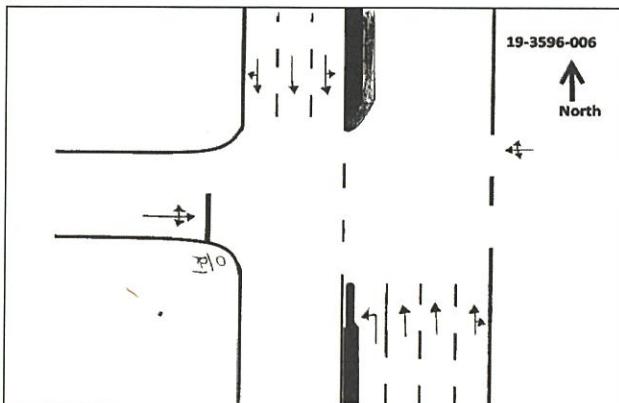
PROJECT ID: 19-03596-006
DATE: 09/17/2019



Peak-Hour: 07:15 AM - 08:15 AM
Peak 15-Minute: 07:15 AM - 07:30 AM

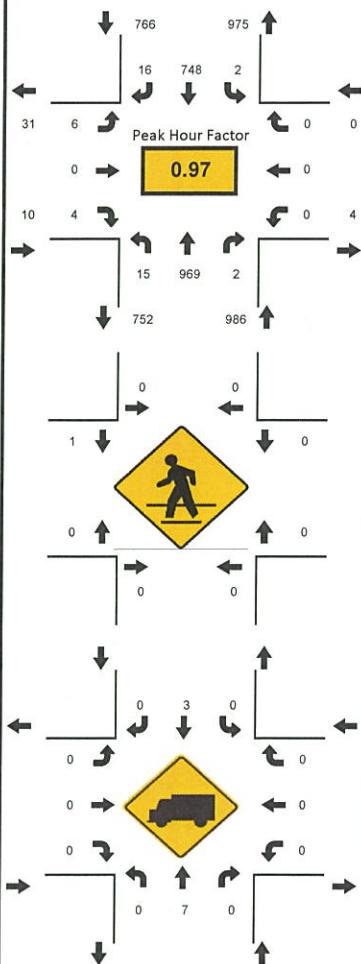


National Data & Surveying Services



LOCATION: 113th St & 62nd Ave
CITY/STATE: Seminole, FL

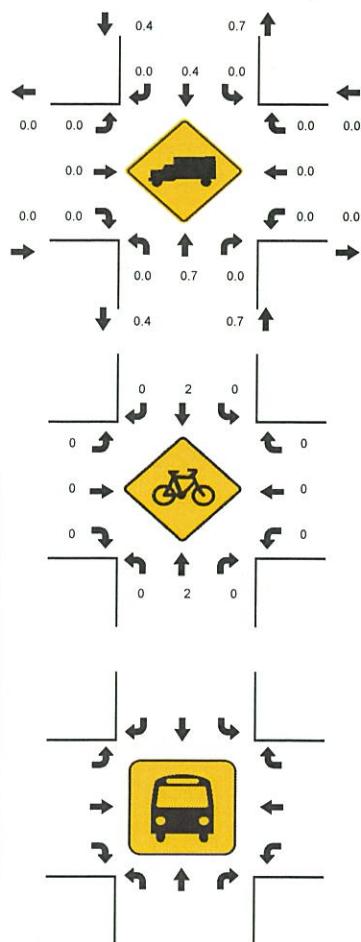
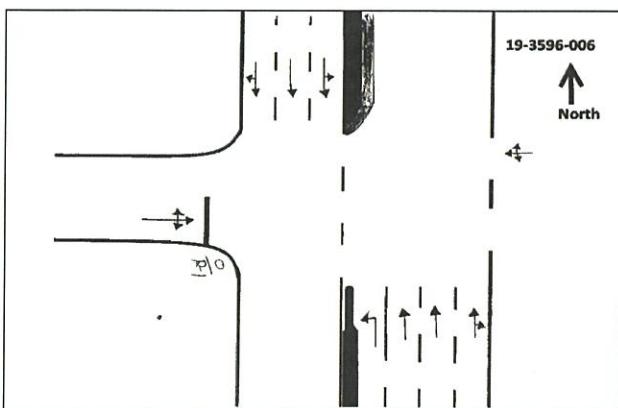
PROJECT ID: 19-03596-006
DATE: 09/17/2019



Peak-Hour: 04:45 PM - 05:45 PM
Peak 15-Minute: 05:15 PM - 05:30 PM



National Data & Surveying Services





National Data & Surveying Services

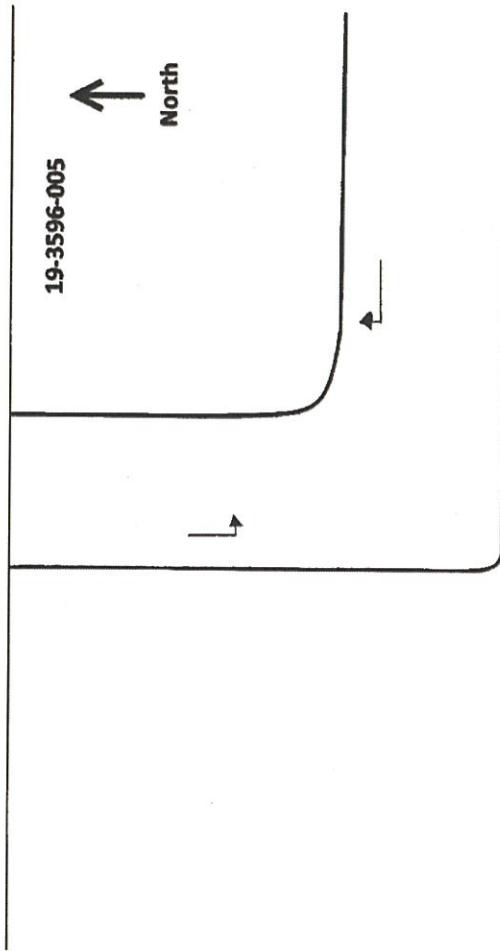
N1

N/S Street: Evergreen Ave N

Speed: 25 MPH

E/W Street: Irving Ave

Speed: 25 MPH



Site Code: 19-3596-005

Date: 9/17/2019

Weather: Sunny

City: Seminole

County: Pinellas

Count Times: 07:00 – 09:00

16:00 - 18:00

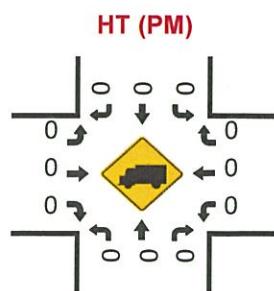
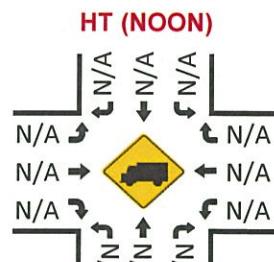
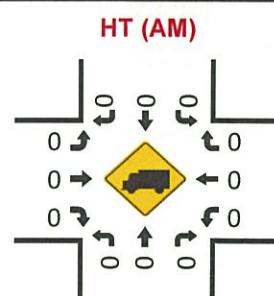
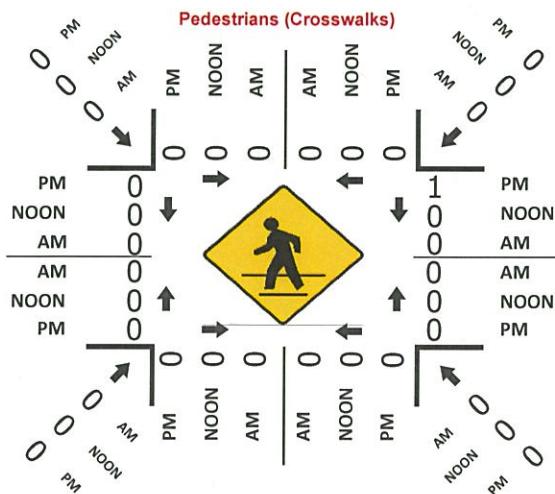
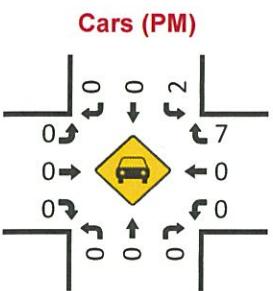
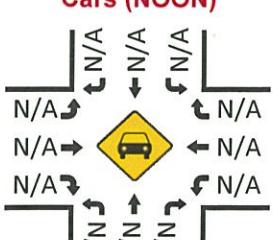
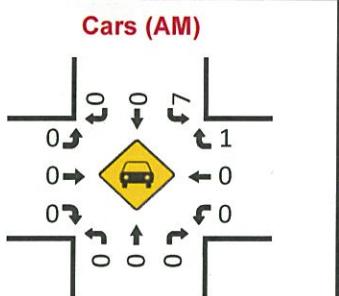
Control: No Control

Evergreen Ave N & Irving Ave

Peak Hour Turning Movement Count

ID: 19-03596-005
City: Seminole

Evergreen Ave N											
SOUTHBOUND											
PEAK HOURS	07:00 AM - 08:00 AM			07:00 AM - 09:00 AM							
	AM	0	0		7	0	1	AM			
	NOON	0	0		0	0	0	NOON			
NONE			NONE								
PEAK HOURS	05:00 PM - 06:00 PM			04:00 PM - 06:00 PM	PM	0	0	2	0	7	PM
	AM	0	0		0	0	0	0			
	NOON	0	0		0	0	0	0			
Irving Ave EASTBOUND	AM	0	0	0	0	0	0	0	0	0	0
	NOON	0	0	0	0	0	0	0	0	0	0
	PM	0	0	0	0	0	0	0	0	0	0
					0	0	0	0	0	0	0
					0	0	0	0	0	0	0
					0	0	0	0	0	0	0
Irving Ave WESTBOUND	AM	0	0	0	0	0	0	0	0	0	0
	NOON	0	0	0	0	0	0	0	0	0	0
	PM	0	0	0	0	0	0	0	0	0	0
					0	0	0	0	0	0	0
					0	0	0	0	0	0	0
					0	0	0	0	0	0	0
CONTROL			TEV			PHF			0.50		
			AM			NOON			PM		
			8			0			9		
			0.50			0.56					



National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & Irving Ave
 City: Seminole
 Control:

Project ID: 19-03596-005
 Date: 9/17/2019

Total

NS/EW Streets:	Evergreen Ave N								Irving Ave							
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
	AM	0	0	0	0	0	0	0	EL	FT	ER	EU	WL	WT	WR	WU
7:00 AM	0	0	0	0	0	2	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	3	0	0	0	0	0	0	0	0	1	1
7:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	4
8:00 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	0	1
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	2
8:30 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	1	1
8:45 AM	0	0	0	0	0	1	0	0	0	0	0	0	0	0	2	2
TOTAL VOLUMES :	0	0	0	0	0	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR
APPROACH %'s :						10	0.00%	0	0.00%	0	0	0	0	0.00%	0.00%	0.00%
PEAK HR :	0	0	0	0	0	7	0	0	0	0	0	0	0	6	0	0
PEAK HR VOL :	0.000	0.000	0.000	0.000	0.000	0.583	0.000	0.000	0.000	0.000	0.000	0.000	0.000	100.00%	100.00%	0.00%
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.583	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.250	0.250	0.500

NS/EW Streets:	Evergreen Ave N								Irving Ave							
	NORTHBOUND				SOUTHBOUND				EASTBOUND				WESTBOUND			
	PM	0	0	0	0	0	0	0	EL	ET	ER	EU	WL	WT	WR	WU
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	1	1
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	1	0	0	0	0	0	0	0	0	2	0	3
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	3	4
5:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0	1	0	1
TOTAL VOLUMES :	0	0	0	0	SL	ST	SR	SU	EL	ET	ER	EU	WL	WT	WR	WU
APPROACH %'s :					2	0.00%	0	0.00%	0	0	0	0	0.00%	0.00%	100.00%	0.00%
PEAK HR :	0	0	0	0	0.500	0.000	0.000	0.000	0.000	0	0	0	0	1	0	1
PEAK HR VOL :	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0	0	0	0.000	0.000	0.583	0.000
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.500	0.000	0.000	0.000	0.000	0	0	0	0.000	0.000	0.563	0.000

National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & Irving Ave
City: Seminole
Control: 0

National Data & Surveying Services

Location: Evergreen Ave N & Irving Ave
City: Seminole
Control: 0

Intersection Turning Movement Count

Project ID: 19-03596-005
Date: 9/17/2019

NS/EW Streets:	Evergreen Ave N				Evergreen Ave N				Irving Ave				Irving Ave			
	NORTHBOUND		SOUTHBOUND		EASTBOUND		WESTBOUND		WL		WR		WL		WR	
	NL	NT	NR	NU	SL	ST	SU	EL	EU	ER	ET	WL	WT	WL	WT	TOTAL
AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %'s:	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK HR VOL :	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0	0
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0
PEAK HR :	07:00 AM - 08:00 AM				0				0				0			

National Data & Surveying Services

Location: Evergreen Ave N & Irving Ave
City: Seminole
Control: 0

Intersection Turning Movement Count

Project ID: 19-03596-005
Date: 9/17/2019

NS/EW Streets:		Evergreen Ave N			Evergreen Ave N			Irving Ave			Irving Ave		
AM		NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND		
NL	NT	NR	0	0	SL	ST	SU	0	0	WL	WT	WR	WU
7:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %/s :	0	0	NR	NU	SL	ST	SU	EL	ET	EU	WL	WT	WR
PEAK HR:	07:00 AM - 08:00 AM			0	0	0	0	0	0	0	0	0	0
PEAK HR VOL :	0	0	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PEAK HR FACTOR :	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000	0.000
PM		NORTHBOUND			SOUTHBOUND			EASTBOUND			WESTBOUND		
NL	NT	NR	0	0	SL	ST	SU	0	0	0	WL	WT	WR
4:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:30 PM	0	0	0	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	1	0	0	0	0	0	0	0	0
TOTAL VOLUMES : APPROACH %/s :	0	0	NR	NU	SL	ST	SU	EL	ET	EU	WL	WT	WR
PEAK HR:	05:00 PM - 06:00 PM			0	100.00%	0.00%	0.00%	0	0	0	0	0	0
PEAK HR VOL :	0	0	0.000	0.000	0.250	0.000	0.000	0.250	0.000	0.000	0.000	0.000	0.000
PEAK HR FACTOR :	0.00	0.00	0.00	0.00	0.25	0.00	0.00	0.25	0.00	0.00	0.00	0.00	0.00

National Data & Surveying Services

Intersection Turning Movement Count

Location: Evergreen Ave N & Irving Ave

City: Seminole

Project ID: 19-03596-005

Date: 9/17/2019

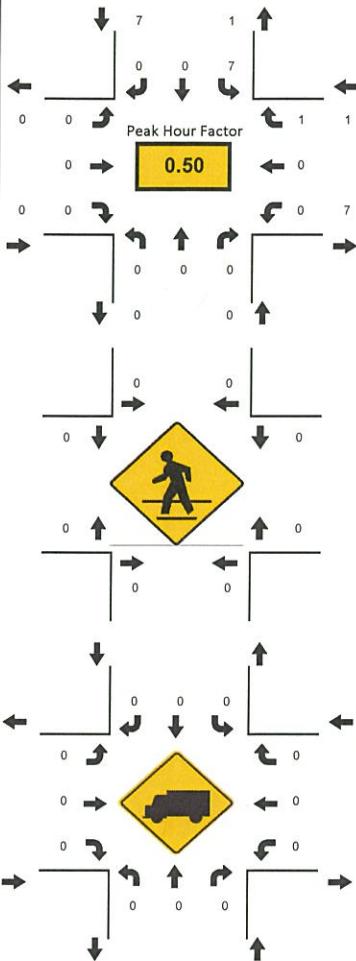
Pedestrians (Crosswalks)

NS/EW Streets:	Evergreen Ave N		Evergreen Ave N		Irving Ave		Irving Ave			
	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG			
AM	WB	EB	WB	EB	WB	NB	SB	NB	SB	TOTAL
7:00 AM	0	0	0	0	0	0	0	0	0	0
7:15 AM	0	0	0	0	0	0	0	0	0	0
7:30 AM	0	0	0	0	0	0	0	0	0	0
7:45 AM	0	0	0	0	0	0	0	0	0	0
8:00 AM	0	0	0	0	0	0	0	0	0	0
8:15 AM	0	0	0	0	0	0	0	0	0	0
8:30 AM	0	0	0	0	0	0	0	0	0	0
8:45 AM	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	0	WB	EB	WB	NB	SB	NB	SB	TOTAL	0
APPROACH %'s :	0	0	0	0	0	0	0	0	0	0
PEAK HR :	07:00 AM - 08:00 AM									
PEAK HR VOL :	0	0	0	0	0	0	0	0	TOTAL	0
PEAK HR FACTOR :										

PM	NORTH LEG		SOUTH LEG		EAST LEG		WEST LEG		TOTAL	
	WB	EB	WB	EB	WB	NB	SB	NB	SB	
4:00 PM	0	0	0	0	0	0	0	0	0	0
4:15 PM	0	0	0	0	0	0	0	0	0	0
4:30 PM	0	0	0	0	0	0	0	0	0	0
4:45 PM	0	0	0	0	0	0	0	0	0	0
5:00 PM	0	0	0	0	0	0	0	0	0	0
5:15 PM	0	0	0	0	0	0	1	0	0	1
5:30 PM	0	0	0	0	0	0	0	0	0	0
5:45 PM	0	0	0	0	0	0	0	0	0	0
TOTAL VOLUMES :	0	WB	EB	WB	NB	SB	NB	SB	TOTAL	1
APPROACH %'s :										
PEAK HR :	05:00 PM - 06:00 PM									
PEAK HR VOL :	0	0	0	0	0	1	0	0	TOTAL	1
PEAK HR FACTOR :										
					0.00%	100.00%	0.250	0.250		0.250

LOCATION: Evergreen Ave N & Irving Ave
CITY/STATE: Seminole, FL

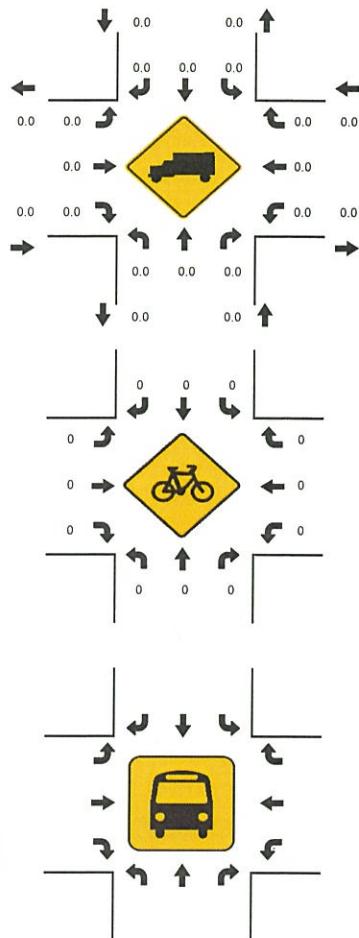
PROJECT ID: 19-03596-005
DATE: 09/17/2019



Peak-Hour: 07:00 AM - 08:00 AM
Peak 15-Minute: 07:30 AM - 07:45 AM



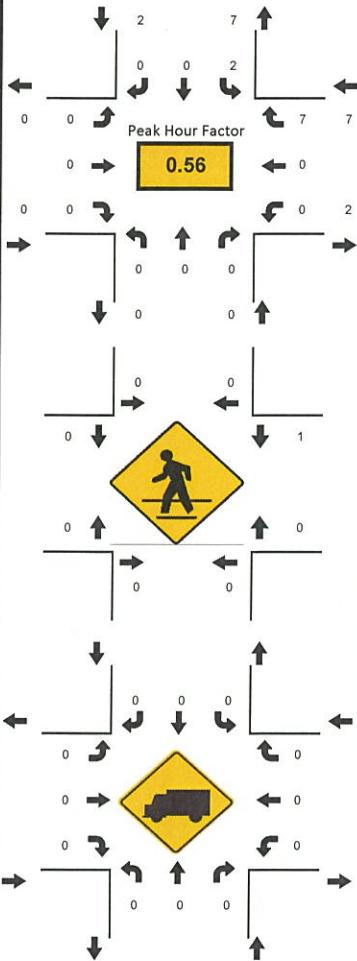
National Data & Surveying Services



15-Min Count Period Beginning At	Evergreen Ave N Northbound					Evergreen Ave N Southbound					Irving Ave Eastbound					Irving Ave Westbound					Total	Hourly Total
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*		
	07:00 AM	0	0	0		2	0	0	0		0	0	0	0		0	0	0	0		2	8
07:15 AM	0	0	0	0		1	0	0	0		0	0	0	0		0	0	0	0		1	8
07:30 AM	0	0	0	0		3	0	0	0		0	0	0	0		0	0	1	0		4	8
07:45 AM	0	0	0	0		1	0	0	0		0	0	0	0		0	0	0	0		1	6
08:00 AM	0	0	0	0		1	0	0	0		0	0	0	0		0	0	1	0		2	8
08:15 AM	0	0	0	0		0	0	0	0		0	0	0	0		0	0	1	0		1	6
08:30 AM	0	0	0	0		1	0	0	0		0	0	0	0		0	0	1	0		2	5
08:45 AM	0	0	0	0		1	0	0	0		0	0	0	0		0	0	0	2		3	3
Peak 15-Min Flowrates	Northbound					Southbound					Eastbound					Westbound						
	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Left	Thru	Rgt	U	R*	Total	
All Vehicles	0	0	0	0		12	0	0	0		0	0	0	0		0	0	4	0		16	
Heavy Trucks	0	0	0			0	0	0			0	0	0			0	0	0			0	
Pedestrians	0					0					0					0					0	
Bicycles	0					0					0					0					0	
Railroad	0					0					0					0		0	0		0	
Stopped Buses	0					0					0					0		0	0		0	

LOCATION: Evergreen Ave N & Irving Ave
CITY/STATE: Seminole, FL

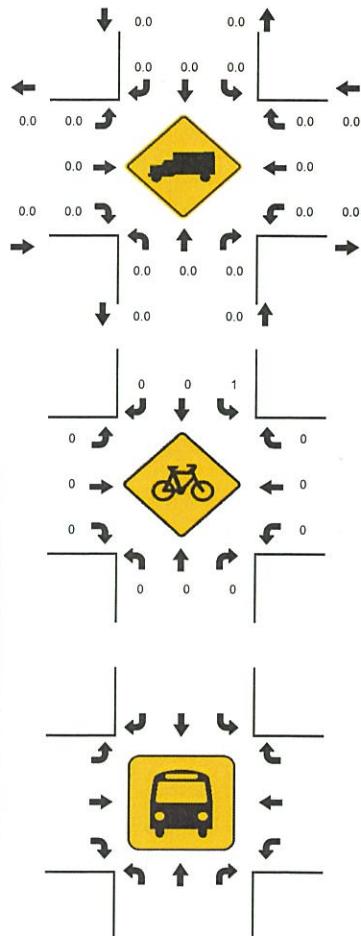
PROJECT ID: 19-03596-005
DATE: 09/17/2019



Peak-Hour: 05:00 PM - 06:00 PM
Peak 15-Minute: 05:15 PM - 05:30 PM



National Data & Surveying Services



FDOT PEAK SEASON FACTOR



LINCKS & ASSOCIATES, INC.

2018 PEAK SEASON FACTOR CATEGORY REPORT - REPORT TYPE: ALL
 CATEGORY: 1500 PINELLAS COUNTYWIDE

MOCF: 0.93
 PSCF

WEEK	DATES	SF	
1	01/01/2018 - 01/06/2018	1.07	1.15
2	01/07/2018 - 01/13/2018	1.06	1.14
3	01/14/2018 - 01/20/2018	1.04	1.12
4	01/21/2018 - 01/27/2018	1.02	1.10
5	01/28/2018 - 02/03/2018	0.99	1.06
* 6	02/04/2018 - 02/10/2018	0.96	1.03
* 7	02/11/2018 - 02/17/2018	0.93	1.00
* 8	02/18/2018 - 02/24/2018	0.93	1.00
* 9	02/25/2018 - 03/03/2018	0.92	0.99
*10	03/04/2018 - 03/10/2018	0.92	0.99
*11	03/11/2018 - 03/17/2018	0.91	0.98
*12	03/18/2018 - 03/24/2018	0.92	0.99
*13	03/25/2018 - 03/31/2018	0.92	0.99
*14	04/01/2018 - 04/07/2018	0.93	1.00
*15	04/08/2018 - 04/14/2018	0.93	1.00
*16	04/15/2018 - 04/21/2018	0.94	1.01
*17	04/22/2018 - 04/28/2018	0.96	1.03
*18	04/29/2018 - 05/05/2018	0.98	1.05
19	05/06/2018 - 05/12/2018	1.00	1.08
20	05/13/2018 - 05/19/2018	1.02	1.10
21	05/20/2018 - 05/26/2018	1.01	1.09
22	05/27/2018 - 06/02/2018	0.99	1.06
23	06/03/2018 - 06/09/2018	0.98	1.05
24	06/10/2018 - 06/16/2018	0.97	1.04
25	06/17/2018 - 06/23/2018	0.98	1.05
26	06/24/2018 - 06/30/2018	0.98	1.05
27	07/01/2018 - 07/07/2018	0.99	1.06
28	07/08/2018 - 07/14/2018	1.00	1.08
29	07/15/2018 - 07/21/2018	1.00	1.08
30	07/22/2018 - 07/28/2018	1.01	1.09
31	07/29/2018 - 08/04/2018	1.02	1.10
32	08/05/2018 - 08/11/2018	1.02	1.10
33	08/12/2018 - 08/18/2018	1.03	1.11
34	08/19/2018 - 08/25/2018	1.04	1.12
35	08/26/2018 - 09/01/2018	1.06	1.14
36	09/02/2018 - 09/08/2018	1.07	1.15
37	09/09/2018 - 09/15/2018	1.08	1.16
38	09/16/2018 - 09/22/2018	1.08	1.16
39	09/23/2018 - 09/29/2018	1.07	1.15
40	09/30/2018 - 10/06/2018	1.06	1.14
41	10/07/2018 - 10/13/2018	1.05	1.13
42	10/14/2018 - 10/20/2018	1.05	1.13
43	10/21/2018 - 10/27/2018	1.05	1.13
44	10/28/2018 - 11/03/2018	1.05	1.13
45	11/04/2018 - 11/10/2018	1.04	1.12
46	11/11/2018 - 11/17/2018	1.04	1.12
47	11/18/2018 - 11/24/2018	1.05	1.13
48	11/25/2018 - 12/01/2018	1.06	1.14
49	12/02/2018 - 12/08/2018	1.06	1.14
50	12/09/2018 - 12/15/2018	1.07	1.15
51	12/16/2018 - 12/22/2018	1.06	1.14
52	12/23/2018 - 12/29/2018	1.05	1.13
53	12/30/2018 - 12/31/2018	1.04	1.12

* PEAK SEASON

25-FEB-2019 16:26:29

830UPD

7_1500_PKSEASON.TXT

SIGNAL TIMINGS



LINCKS & ASSOCIATES, INC.

Intersection 870

Report Date: 10/15/2019

Run Time: 06:36 AM

Main Street: 113TH ST

Side Street: 66TH AVE N

Jurisdiction: COUNTY

Section #: 62 MIST

Comm. Addrs: IP: 10.198.100.138 Gateway: 10.198.100.254 Subnet: 255.255.255.0

Pre-empt: Y

Phase #	Street Name	Direction		Left Turn Type
1	113TH ST.	NB	LT	Protected/Permitted
2	113TH ST. N.	SB		
3				
4				
5				
6	113TH ST. N.	NB		
7				
8	66TH AVE. N.	EB/WB		

Timing Plan 1 (MM,2,1)

PHASE	1	2	3	4	5	6	7	8
Min. Green	7	20				20		7
Walk		7				7		7
Ped Clr		11				11		22
Veh Ext	2	4				4		3
Yellow Clr	4.4	4.4				4.4		3.7
Red Clr	2.1	2.2				2.2		3.6
Max 1	13	45				45		25
Max 2								
Max 3								
Walk 2								
Ped Clr 2								
Lock Det								
Veh Recall								
Ped Recall								
Max Recall		X				X		
CNA 1		X				X		
Phase In Use	X	X				X		X
Flash		Y				Y		R
Delay Det.								8

Last Timing Change Date: 02/11/2019	Database Modified: 02/12/2019
Technician Initials:	Control Room Pers. Initials:

COORD PATTERNS (CYCLE / OFFSET) (MM,3,2)

Cycle	Sec.
1	105
2	90
3	105
4	85

Offset	Sec. / %
1	75
2	23
3	100
4	75

COORD PATTERNS

	Ph 1 Sec / %	Ph 2 Sec / %	Ph 3 Sec / %	Ph 4 Sec / %	Ph 5 Sec / %	Ph 6 Sec / %	Ph 7 Sec / %	Ph 8 Sec / %
PATTERN 1	18	52	0	0	0	70	0	35
PATTERN 2	16	43				59	0	31
PATTERN 3	16	53	0	0	0	69	0	36
PATTERN 4	17	37	0	0	0	54	0	31

DAY PLANS (MM,5,3)

Event	Action Plan #	Time	Action	On/Off
DAY PLAN1				
1	1	0600		
2	2	0930		
3	3	1430		
4	4	1830		
5	100	2100	FRE	ON
DAY PLAN2				
1	2	0800		
2	100	2100	FRE	ON

Notes:

INTERSECTION ANALYSIS



LINCKS & ASSOCIATES, INC.

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/17/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	121	0	182	0	0	0	106	725	0	0	996	72
Future Volume (vph)	121	0	182	0	0	0	106	725	0	0	996	72
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	142		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.91	0.91	0.91
Frt												0.990
Flt Protected								0.950				
Satd. Flow (prot)	0	1678	0	0	1863	0	1770	5085	0	0	5034	0
Flt Permitted		0.869					0.175					
Satd. Flow (perm)	0	1488	0	0	1863	0	326	5085	0	0	5034	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		98										15
Link Speed (mph)		30			30			30				30
Link Distance (ft)		709			271			694				648
Travel Time (s)		16.1			6.2			15.8				14.7
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	133	0	200	0	0	0	116	797	0	0	1095	79
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	333	0	0	0	0	116	797	0	0	1174	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)	0				0			12				12
Link Offset(ft)	0				0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru										
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94				94
Detector 2 Size(ft)		6			6			6				6
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex				Cl+Ex
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0				0.0
Turn Type	Perm	NA				pm+pt		NA				NA
Protected Phases		4			8		5	2				6
Permitted Phases		4			8		2					6

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/17/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	25.3	25.3		25.3	25.3		9.5	24.6		24.6	24.6	
Total Split (s)	31.0	31.0		31.0	31.0		16.0	59.0		43.0	43.0	
Total Split (%)	34.4%	34.4%		34.4%	34.4%		17.8%	65.6%		47.8%	47.8%	
Maximum Green (s)	23.7	23.7		23.7	23.7		11.5	52.4		36.4	36.4	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.5	4.4		4.4	4.4	
All-Red Time (s)	3.6	3.6		3.6	3.6		1.0	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		7.3			7.3		4.5	6.6			6.6	
Lead/Lag							Lead			Lag	Lag	
Lead-Lag Optimize?							Yes			Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0		0	0	
Act Effct Green (s)		19.2					59.0	56.9			46.6	
Actuated g/C Ratio		0.21					0.66	0.63			0.52	
v/c Ratio		0.85					0.34	0.25			0.45	
Control Delay		42.9					9.6	8.0			15.9	
Queue Delay		0.0					0.0	0.0			0.0	
Total Delay		42.9					9.6	8.0			15.9	
LOS		D					A	A			B	
Approach Delay		42.9						8.2			15.9	
Approach LOS		D					A				B	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 23 (26%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.85

Intersection Signal Delay: 16.7

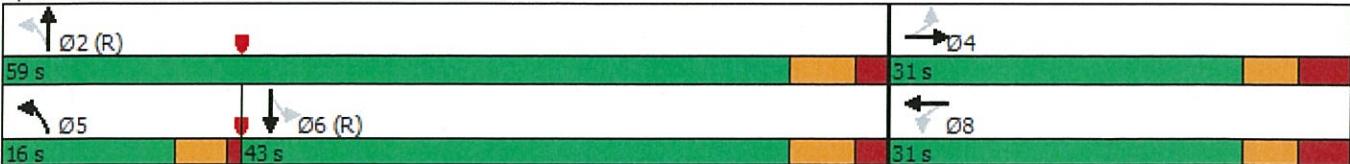
Intersection LOS: B

Intersection Capacity Utilization 69.8%

ICU Level of Service C

Analysis Period (min) 15

Splits and Phases: 3: 113TH STR & 66TH Ave N



Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/17/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	46	0	91	0	0	1	182	1002	1	1	864	91
Future Volume (vph)	46	0	91	0	0	1	182	1002	1	1	864	91
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	142		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.91	0.91	0.91
Frt		0.910			0.865						0.986	
Flt Protected		0.983					0.950					
Satd. Flow (prot)	0	1666	0	0	1611	0	1770	5085	0	0	5014	0
Flt Permitted		0.888					0.255				0.939	
Satd. Flow (perm)	0	1505	0	0	1611	0	475	5085	0	0	4708	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		93			112						22	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		709			271			694			648	
Travel Time (s)		16.1			6.2			15.8			14.7	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	47	0	93	0	0	1	186	1022	1	1	882	93
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	140	0	0	1	0	186	1023	0	0	976	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru										
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		NA		pm+pt	NA		Perm	NA		
Protected Phases		4			8		5	2			6	
Permitted Phases		4			8		2			6		

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/17/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	25.3	25.3		25.3	25.3		9.5	24.6		24.6	24.6	
Total Split (s)	36.0	36.0		36.0	36.0		16.0	69.0		53.0	53.0	
Total Split (%)	34.3%	34.3%		34.3%	34.3%		15.2%	65.7%		50.5%	50.5%	
Maximum Green (s)	28.7	28.7		28.7	28.7		11.5	62.4		46.4	46.4	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.5	4.4		4.4	4.4	
All-Red Time (s)	3.6	3.6		3.6	3.6		1.0	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		7.3			7.3		4.5	6.6			6.6	
Lead/Lag							Lead			Lag		Lag
Lead-Lag Optimize?							Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0		0	0	
Act Effect Green (s)		9.4			9.4		83.8	81.7			69.0	
Actuated g/C Ratio		0.09			0.09		0.80	0.78			0.66	
v/c Ratio		0.64			0.00		0.39	0.26			0.31	
Control Delay		30.6			0.0		5.2	3.7			8.5	
Queue Delay		0.0			0.0		0.0	0.0			0.0	
Total Delay		30.6			0.0		5.2	3.7			8.5	
LOS		C			A		A	A			A	
Approach Delay		30.6						3.9			8.5	
Approach LOS		C						A			A	

Intersection Summary

Area Type: Other

Cycle Length: 105

Actuated Cycle Length: 105

Offset: 100 (95%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.64

Intersection Signal Delay: 7.5

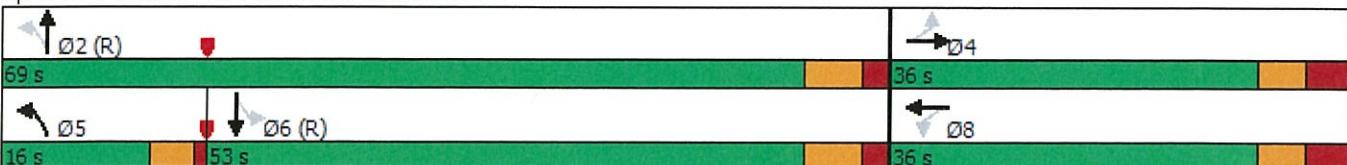
Intersection LOS: A

Intersection Capacity Utilization 70.0%

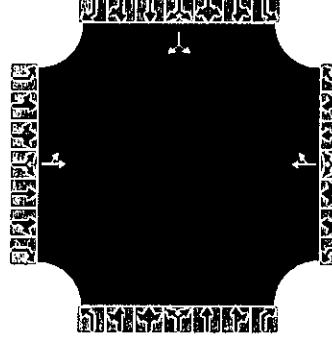
ICU Level of Service C

Analysis Period (min) 15

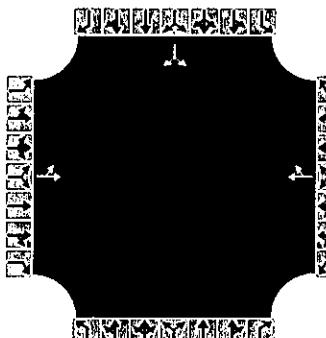
Splits and Phases: 3: 113TH STR & 66TH Ave N



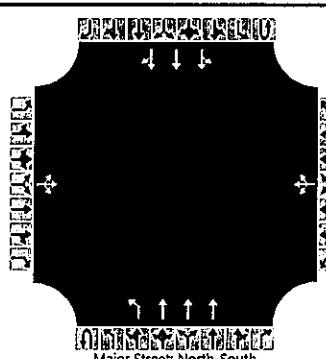
HCS7 All-Way Stop Control Report

General Information			Site Information																			
Analyst			Intersection																			
Agency/Co.			Jurisdiction																			
Date Performed	10/17/2019		East/West Street			66th Ave North																
Analysis Year	2019		North/South Street			116th St																
Analysis Time Period (hrs)	0.25		Peak Hour Factor			0.75																
Time Analyzed	AM Peak																					
Project Description	Background																					
Lanes																						
																						
Vehicle Volume and Adjustments																						
Approach	Eastbound			Westbound			Northbound		Southbound													
Movement	L	T	R	L	T	R	L	T	R	L	T	R										
Volume	8	82			10	66				277		9										
% Thrus in Shared Lane																						
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3										
Configuration	LT			TR						LR												
Flow Rate, v (veh/h)	120			101						381												
Percent Heavy Vehicles	2			2						2												
Departure Headway and Service Time																						
Initial Departure Headway, hd (s)	3.20			3.20					3.20													
Initial Degree of Utilization, x	0.107			0.090					0.339													
Final Departure Headway, hd (s)	5.05			4.54					4.66													
Final Degree of Utilization, x	0.168			0.128					0.494													
Move-Up Time, m (s)	2.0			2.0					2.0													
Service Time, ts (s)	3.05			2.54					2.66													
Capacity, Delay and Level of Service																						
Flow Rate, v (veh/h)	120			101					381													
Capacity	713			792					772													
95% Queue Length, Q ₉₅ (veh)	0.6			0.4					2.8													
Control Delay (s/veh)	9.1			8.2					12.1													
Level of Service, LOS	A			A					B													
Approach Delay (s/veh)	9.1		8.2							12.1												
Approach LOS	A		A							B												
Intersection Delay, s/veh LOS	10.9								B													

HCS7 All-Way Stop Control Report

General Information				Site Information																										
Analyst				Intersection																										
Agency/Co.				Jurisdiction																										
Date Performed	10/17/2019			East/West Street				66th Ave North																						
Analysis Year	2019			North/South Street				116th St																						
Analysis Time Period (hrs)	0.25			Peak Hour Factor				0.98																						
Time Analyzed	PM Peak																													
Project Description	Background																													
Lanes																														
																														
Vehicle Volume and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	L	T	R	L	T	R	L	T	R	L	T																			
Volume	9	33			82	168				92																				
% Thrus in Shared Lane																														
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2																			
Configuration	LT			TR						LR																				
Flow Rate, v (veh/h)	43			255						111																				
Percent Heavy Vehicles	2			2						2																				
Departure Headway and Service Time																														
Initial Departure Headway, hd (s)	3.20			3.20						3.20																				
Initial Degree of Utilization, x	0.038			0.227						0.099																				
Final Departure Headway, hd (s)	4.49			3.85						4.61																				
Final Degree of Utilization, x	0.053			0.273						0.143																				
Move-Up Time, m (s)	2.0			2.0						2.0																				
Service Time, ts (s)	2.49			1.85						2.61																				
Capacity, Delay and Level of Service																														
Flow Rate, v (veh/h)	43			255						111																				
Capacity	802			935						780																				
95% Queue Length, Q ₉₅ (veh)	0.2			1.1						0.5																				
Control Delay (s/veh)	7.7			8.3						8.4																				
Level of Service, LOS	A			A						A																				
Approach Delay (s/veh)	7.7			8.3																										
Approach LOS	A			A																										
Intersection Delay, s/veh LOS	8.3					A																								

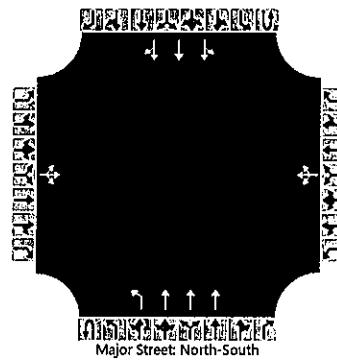
HCS7 Two-Way Stop-Control Report

General Information				Site Information																															
Analyst				Intersection																															
Agency/Co.				Jurisdiction																															
Date Performed	10/23/2019				East/West Street		62nd Avenue																												
Analysis Year	2019				North/South Street		113th Street																												
Time Analyzed	AM Peak				Peak Hour Factor		0.91																												
Intersection Orientation	North-South				Analysis Time Period (hrs)		0.25																												
Project Description	Background																																		
Lanes																																			
 Major Street: North-South																																			
Vehicle Volumes and Adjustments																																			
Approach	Eastbound				Westbound				Northbound				Southbound																						
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U																						
Priority		10	11	12		7	8	9	1U	1	2	3	4U																						
Number of Lanes		0	1	0		0	1	0	0	1	3	0	0																						
Configuration			LTR				LTR			L	T		LT																						
Volume (veh/h)		13	0	19		0	0	1	0	4	739		1																						
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3				3																						
Proportion Time Blocked																																			
Percent Grade (%)	0			0																															
Right Turn Channelized																																			
Median Type Storage	Undivided																																		
Critical and Follow-up Headways																																			
Base Critical Headway (sec)		6.4	6.5	7.1		6.4	6.5	7.1		5.3			5.3																						
Critical Headway (sec)		6.46	6.56	7.16		6.46	6.56	7.16		5.36			5.36																						
Base Follow-Up Headway (sec)		3.8	4.0	3.9		3.8	4.0	3.9		3.1			3.1																						
Follow-Up Headway (sec)		3.83	4.03	3.93		3.83	4.03	3.93		3.13			3.13																						
Delay, Queue Length, and Level of Service																																			
Flow Rate, v (veh/h)			35				1			4			1																						
Capacity, c (veh/h)			179				506			278			479																						
v/c Ratio			0.20				0.00			0.02			0.00																						
95% Queue Length, Q ₉₅ (veh)			0.7				0.0			0.0			0.0																						
Control Delay (s/veh)			30.0				12.1			18.2			12.5																						
Level of Service (LOS)			D				B			C			B																						
Approach Delay (s/veh)	30.0			12.1			0.1			0.0																									
Approach LOS	D			B																															

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst		Intersection	
Agency/Co.		Jurisdiction	
Date Performed	10/23/2019	East/West Street	62nd Avenue
Analysis Year	2019	North/South Street	113th Street
Time Analyzed	PM Peak	Peak Hour Factor	0.97
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Background		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	3	0	0	0	3	0
Configuration			LTR				LTR			L	T			LT	T	TR
Volume (veh/h)		7	0	5		0	0	0	0	17	1124			2	868	19
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3				3		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized																
Median Type Storage		Undivided														

Critical and Follow-up Headways

Base Critical Headway (sec)		6.4	6.5	7.1		6.4	6.5	7.1		5.3				5.3		
Critical Headway (sec)		6.46	6.56	7.16		6.46	6.56	7.16		5.36				5.36		
Base Follow-Up Headway (sec)		3.8	4.0	3.9		3.8	4.0	3.9		3.1				3.1		
Follow-Up Headway (sec)		3.83	4.03	3.93		3.83	4.03	3.93		3.13				3.13		

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		12			0			18				2				
Capacity, c (veh/h)		197						427				325				
v/c Ratio		0.06						0.04				0.01				
95% Queue Length, Q ₉₅ (veh)		0.2						0.1				0.0				
Control Delay (s/veh)		24.5						13.8				16.1				
Level of Service (LOS)		C						B				C				
Approach Delay (s/veh)	24.5					0.2					0.1					
Approach LOS	C															

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/17/2019

	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	166	0	271	0	0	0	136	725	0	0	996	87
Future Volume (vph)	166	0	271	0	0	0	136	725	0	0	996	87
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	142		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.91	0.91	0.91
Frt		0.916									0.988	
Flt Protected		0.981					0.950					
Satd. Flow (prot)	0	1674	0	0	1863	0	1770	5085	0	0	5024	0
Flt Permitted		0.875					0.152					
Satd. Flow (perm)	0	1493	0	0	1863	0	283	5085	0	0	5024	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		98									19	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		709			271			694			648	
Travel Time (s)		16.1			6.2			15.8			14.7	
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	182	0	298	0	0	0	149	797	0	0	1095	96
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	480	0	0	0	0	149	797	0	0	1191	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru										
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA				pm+pt		NA			NA	
Protected Phases		4			8		5	2			6	
Permitted Phases		4			8		2			6		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	25.3	25.3		25.3	25.3		9.5	24.6		24.6	24.6	
Total Split (s)	31.0	31.0		31.0	31.0		16.0	59.0		43.0	43.0	
Total Split (%)	34.4%	34.4%		34.4%	34.4%		17.8%	65.6%		47.8%	47.8%	
Maximum Green (s)	23.7	23.7		23.7	23.7		11.5	52.4		36.4	36.4	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.5	4.4		4.4	4.4	
All-Red Time (s)	3.6	3.6		3.6	3.6		1.0	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		7.3			7.3		4.5	6.6			6.6	
Lead/Lag							Lead			Lag		Lag
Lead-Lag Optimize?							Yes			Yes		Yes
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0		0	0	
Act Effct Green (s)		23.7					54.5	52.4			39.1	
Actuated g/C Ratio		0.26					0.61	0.58			0.43	
v/c Ratio		1.03					0.47	0.27			0.54	
Control Delay		78.4					12.8	9.6			19.9	
Queue Delay		0.0					0.0	0.0			0.0	
Total Delay		78.4					12.8	9.6			19.9	
LOS		E					B	A			B	
Approach Delay		78.4						10.1			19.9	
Approach LOS		E						B			B	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 23 (26%), Referenced to phase 2:NBT and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 1.03

Intersection Signal Delay: 27.1

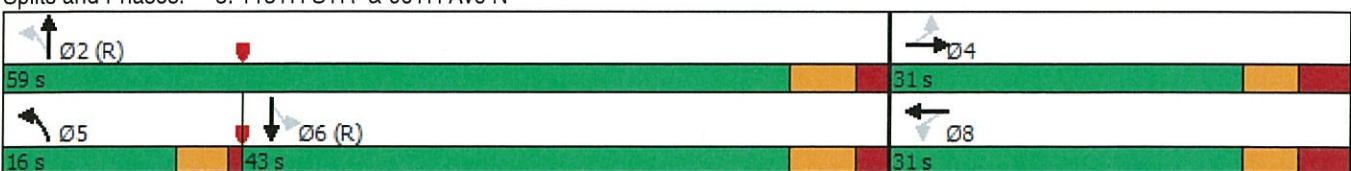
Intersection LOS: C

Intersection Capacity Utilization 78.1%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: 113TH STR & 66TH Ave N



Queuing and Blocking Report
Option A-Background Plus Project

10/17/2019

Intersection: 3: 113TH STR & 66TH Ave N

Movement	EB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	T	T	TR	LT	T	TR
Maximum Queue (ft)	355	97	155	123	54	262	225	121
Average Queue (ft)	234	60	104	65	17	189	141	59
95th Queue (ft)	401	103	168	126	58	269	236	126
Link Distance (ft)	651		666	666	666	620	620	620
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)			142					
Storage Blk Time (%)				2				
Queuing Penalty (veh)					2			

Network Summary

Network wide Queuing Penalty: 2



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	75	0	150	0	0	1	283	1002	1	1	864	141
Future Volume (vph)	75	0	150	0	0	1	283	1002	1	1	864	141
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	142		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.91	0.91	0.91
Frt		0.910				0.865						0.979
Flt Protected		0.984					0.950					
Satd. Flow (prot)	0	1668	0	0	1611	0	1770	5085	0	0	4979	0
Flt Permitted		0.888					0.225					0.939
Satd. Flow (perm)	0	1505	0	0	1611	0	419	5085	0	0	4675	0
Right Turn on Red		Yes				Yes			Yes			Yes
Satd. Flow (RTOR)		94			112							38
Link Speed (mph)		30			30			30				30
Link Distance (ft)		709			271			694				648
Travel Time (s)		16.1			6.2			15.8				14.7
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	77	0	153	0	0	1	289	1022	1	1	882	144
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	230	0	0	1	0	289	1023	0	0	1027	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru										
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		NA		pm+pt	NA		Perm	NA		
Protected Phases		4			8		5	2			6	
Permitted Phases		4			8		2				6	



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	25.3	25.3		25.3	25.3		9.5	24.6		24.6	24.6	
Total Split (s)	36.0	36.0		36.0	36.0		16.0	69.0		53.0	53.0	
Total Split (%)	34.3%	34.3%		34.3%	34.3%		15.2%	65.7%		50.5%	50.5%	
Maximum Green (s)	28.7	28.7		28.7	28.7		11.5	62.4		46.4	46.4	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.5	4.4		4.4	4.4	
All-Red Time (s)	3.6	3.6		3.6	3.6		1.0	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		7.3			7.3		4.5	6.6			6.6	
Lead/Lag							Lead			Lag	Lag	
Lead-Lag Optimize?							Yes			Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0		0	0	
Act Effct Green (s)		15.4			15.4		77.8	75.7			59.4	
Actuated g/C Ratio		0.15			0.15		0.74	0.72			0.57	
v/c Ratio		0.76			0.00		0.63	0.28			0.39	
Control Delay		40.9			0.0		11.4	6.0			14.1	
Queue Delay		0.0			0.0		0.0	0.0			0.0	
Total Delay		40.9			0.0		11.4	6.0			14.1	
LOS		D			A		B	A			B	
Approach Delay		40.9						7.2			14.1	
Approach LOS		D						A			B	

Intersection Summary

Area Type: Other

Cycle Length: 105

Actuated Cycle Length: 105

Offset: 100 (95%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.76

Intersection Signal Delay: 12.9

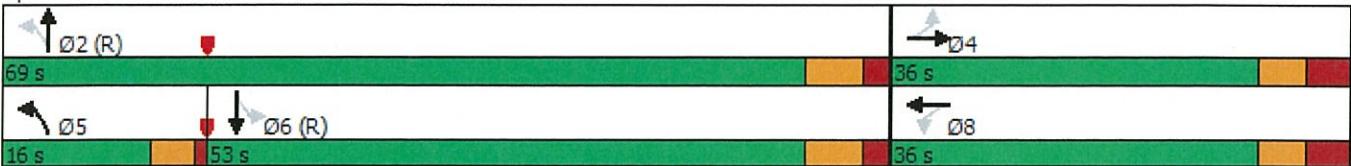
Intersection LOS: B

Intersection Capacity Utilization 76.4%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: 113TH STR & 66TH Ave N



Queuing and Blocking Report
Option A-Background Plus Project

10/17/2019

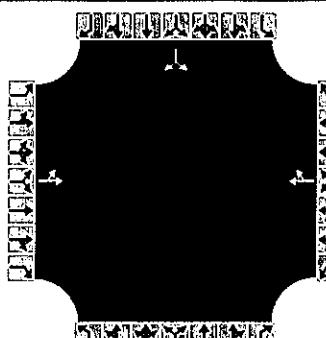
Intersection: 3: 113TH STR & 66TH Ave N

Movement	EB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	LTR	L	T	T	TR	LT	T	TR
Maximum Queue (ft)	242	12	148	204	152	101	242	209	107
Average Queue (ft)	136	2	97	107	79	36	148	100	48
95th Queue (ft)	252	13	162	224	165	100	267	222	105
Link Distance (ft)	651	213		666	666	666	620	620	620
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)				142					
Storage Blk Time (%)				4	2				
Queuing Penalty (veh)				12	6				

Network Summary

Network wide Queuing Penalty: 18

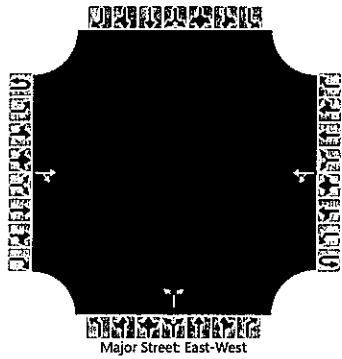
HCS7 All-Way Stop Control Report

General Information			Site Information																			
Analyst			Intersection																			
Agency/Co.			Jurisdiction																			
Date Performed	10/17/2019		East/West Street			66th Ave North																
Analysis Year	2019		North/South Street			116th St																
Analysis Time Period (hrs)	0.25		Peak Hour Factor			0.75																
Time Analyzed	AM Peak																					
Project Description	Option A -Background Plus Project																					
Lanes																						
																						
Vehicle Volume and Adjustments																						
Approach	Eastbound			Westbound			Northbound		Southbound													
Movement	L	T	R	L	T	R	L	T	R	L	T	R										
Volume	23	216			55	66				277		14										
% Thrus in Shared Lane																						
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3										
Configuration	LT			TR						LR												
Flow Rate, v (veh/h)	319			161						388												
Percent Heavy Vehicles	2			2						2												
Departure Headway and Service Time																						
Initial Departure Headway, hd (s)	3.20			3.20					3.20													
Initial Degree of Utilization, x	0.283			0.143					0.345													
Final Departure Headway, hd (s)	5.34			5.25					5.38													
Final Degree of Utilization, x	0.473			0.235					0.580													
Move-Up Time, m (s)	2.0			2.0					2.0													
Service Time, ts (s)	3.34			3.25					3.38													
Capacity, Delay and Level of Service																						
Flow Rate, v (veh/h)	319			161					388													
Capacity	674			686					669													
95% Queue Length, Q ₉₅ (veh)	2.5			0.9					3.7													
Control Delay (s/veh)	13.0			9.9					15.5													
Level of Service, LOS	B			A					C													
Approach Delay (s/veh)	13.0			9.9						15.5												
Approach LOS	B			A						C												
Intersection Delay, s/veh LOS	13.6					B																

HCS7 All-Way Stop Control Report

General Information				Site Information																										
Analyst				Intersection																										
Agency/Co.				Jurisdiction																										
Date Performed	10/17/2019			East/West Street				66th Ave North																						
Analysis Year	2019			North/South Street				116th St																						
Analysis Time Period (hrs)	0.25			Peak Hour Factor				0.98																						
Time Analyzed	PM Peak																													
Project Description	Option A -Background Plus Project																													
Lanes																														
Vehicle Volume and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	L	T	R	L	T	R	L	T	R	L	T																			
Volume	19	121			233	168				92																				
% Thrus in Shared Lane											34																			
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2																			
Configuration	LT			TR						LR																				
Flow Rate, v (veh/h)	143			409						129																				
Percent Heavy Vehicles	2			2						2																				
Departure Headway and Service Time																														
Initial Departure Headway, hd (s)	3.20			3.20						3.20																				
Initial Degree of Utilization, x	0.127			0.364						0.114																				
Final Departure Headway, hd (s)	4.75			4.21						5.13																				
Final Degree of Utilization, x	0.188			0.478						0.183																				
Move-Up Time, m (s)	2.0			2.0						2.0																				
Service Time, ts (s)	2.75			2.21						3.13																				
Capacity, Delay and Level of Service																														
Flow Rate, v (veh/h)	143			409						129																				
Capacity	758			856						701																				
95% Queue Length, Q ₉₅ (veh)	0.7			2.6						0.7																				
Control Delay (s/veh)	8.8			11.0						9.3																				
Level of Service, LOS	A			B						A																				
Approach Delay (s/veh)	8.8			11.0																										
Approach LOS	A			B																										
Intersection Delay, s/veh LOS	10.2					B																								

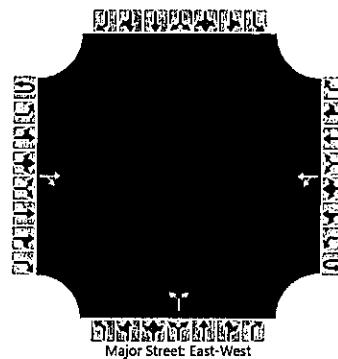
HCS7 Two-Way Stop-Control Report

General Information				Site Information																																						
Analyst				Intersection																																						
Agency/Co.				Jurisdiction																																						
Date Performed	10/18/2019			East/West Street				66th Avenue North																																		
Analysis Year	2019			North/South Street				Project Access A																																		
Time Analyzed	AM Peak			Peak Hour Factor				0.86																																		
Intersection Orientation	East-West			Analysis Time Period (hrs)				0.25																																		
Project Description	Option A-Background Plus Project																																									
Lanes																																										
 Major Street East-West																																										
Vehicle Volumes and Adjustments																																										
Approach	Eastbound				Westbound				Northbound				Southbound																													
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R																										
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12																										
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0																										
Configuration				TR		LT					LR																															
Volume (veh/h)			70	0		50	16			0		149																														
Percent Heavy Vehicles (%)						3				3		3																														
Proportion Time Blocked																																										
Percent Grade (%)										0																																
Right Turn Channelized																																										
Median Type Storage	Undivided																																									
Critical and Follow-up Headways																																										
Base Critical Headway (sec)						4.1				7.1		6.2																														
Critical Headway (sec)						4.13				6.43		6.23																														
Base Follow-Up Headway (sec)						2.2				3.5		3.3																														
Follow-Up Headway (sec)						2.23				3.53		3.33																														
Delay, Queue Length, and Level of Service																																										
Flow Rate, v (veh/h)						58				173																																
Capacity, c (veh/h)						1510				976																																
v/c Ratio						0.04				0.18																																
95% Queue Length, Q ₉₅ (veh)						0.1				0.6																																
Control Delay (s/veh)						7.5				9.5																																
Level of Service (LOS)						A				A																																
Approach Delay (s/veh)				5.7				9.5																																		
Approach LOS								A																																		

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst		Intersection	
Agency/Co.		Jurisdiction	
Date Performed	10/18/2019	East/West Street	66th Avenue North
Analysis Year	2019	North/South Street	Project Access A
Time Analyzed	PM Peak	Peak Hour Factor	0.66
Intersection Orientation	East-West	Analysis Time Period (hrs)	0.25
Project Description	Option A-Background Plus Project		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0	0	0	0	0	0
Configuration				TR		LT				LR						
Volume (veh/h)			39	0		168	71			0		98				
Percent Heavy Vehicles (%)						3				3		3				
Proportion Time Blocked																
Percent Grade (%)										0						
Right Turn Channelized																
Median Type Storage	Undivided															

Critical and Follow-up Headways

Base Critical Headway (sec)					4.1				7.1		6.2					
Critical Headway (sec)						4.13				6.43		6.23				
Base Follow-Up Headway (sec)						2.2				3.5		3.3				
Follow-Up Headway (sec)						2.23				3.53		3.33				

Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)					255				148							
Capacity, c (veh/h)						1538				1004						
v/c Ratio						0.17				0.15						
95% Queue Length, Q ₉₅ (veh)						0.6				0.5						
Control Delay (s/veh)						7.8				9.2						
Level of Service (LOS)						A				A						
Approach Delay (s/veh)					5.9				9.2							
Approach LOS									A							

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/23/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	143	0	212	0	0	0	117	748	0	0	1004	79
Future Volume (vph)	143	0	212	0	0	0	117	748	0	0	1004	79
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	142		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.91	0.91	0.91
Frt					0.919							0.989
Flt Protected							0.950					
Satd. Flow (prot)	0	1678	0	0	1863	0	1770	5085	0	0	5029	0
Flt Permitted							0.160					
Satd. Flow (perm)	0	1486	0	0	1863	0	298	5085	0	0	5029	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		98										17
Link Speed (mph)		30			30			30				30
Link Distance (ft)		709			271			694				648
Travel Time (s)		16.1			6.2			15.8				14.7
Peak Hour Factor	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91	0.91
Adj. Flow (vph)	157	0	233	0	0	0	129	822	0	0	1103	87
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	390	0	0	0	0	129	822	0	0	1190	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		0			0			12				12
Link Offset(ft)		0			0			0				0
Crosswalk Width(ft)		16			16			16				16
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru										
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA				pm+pt		NA			NA	
Protected Phases		4			8		5	2			6	
Permitted Phases		4			8		2			6		

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/23/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	25.3	25.3		25.3	25.3		9.5	24.6		24.6	24.6	
Total Split (s)	31.0	31.0		31.0	31.0		16.0	59.0		43.0	43.0	
Total Split (%)	34.4%	34.4%		34.4%	34.4%		17.8%	65.6%		47.8%	47.8%	
Maximum Green (s)	23.7	23.7		23.7	23.7		11.5	52.4		36.4	36.4	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.5	4.4		4.4	4.4	
All-Red Time (s)	3.6	3.6		3.6	3.6		1.0	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		7.3			7.3		4.5	6.6			6.6	
Lead/Lag							Lead			Lag	Lag	
Lead-Lag Optimize?							Yes			Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0		0	0	
Act Effect Green (s)		21.6					56.6	54.5			41.8	
Actuated g/C Ratio		0.24					0.63	0.61			0.46	
v/c Ratio		0.90					0.40	0.27			0.51	
Control Delay		50.6					11.1	9.0			18.3	
Queue Delay		0.0					0.0	0.0			0.0	
Total Delay		50.6					11.1	9.0			18.3	
LOS		D					B	A			B	
Approach Delay		50.6						9.3			18.3	
Approach LOS		D						A			B	

Intersection Summary

Area Type: Other

Cycle Length: 90

Actuated Cycle Length: 90

Offset: 23 (26%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.90

Intersection Signal Delay: 19.9

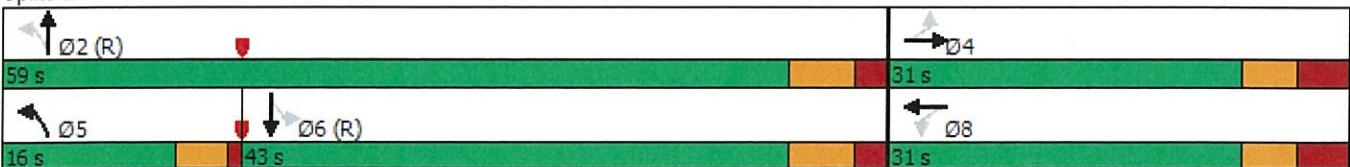
Intersection LOS: B

Intersection Capacity Utilization 73.6%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: 113TH STR & 66TH Ave N



Queuing and Blocking Report

Option B-Background Plus Project

10/23/2019

Intersection: 3: 113TH STR & 66TH Ave N

Movement	EB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	L	T	T	TR	LT	T	TR
Maximum Queue (ft)	276	92	149	115	46	256	217	133
Average Queue (ft)	165	55	98	63	15	177	122	58
95th Queue (ft)	295	97	164	118	46	268	223	133
Link Distance (ft)	651		666	666	666	620	620	620
Upstream Blk Time (%)								
Queuing Penalty (veh)								
Storage Bay Dist (ft)			142					
Storage Blk Time (%)				1				
Queuing Penalty (veh)					1			

Network Summary

Network wide Queuing Penalty: 1

Lanes, Volumes, Timings
3: 113TH STR & 66TH Ave N

10/23/2019



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Lane Configurations												
Traffic Volume (vph)	61	0	110	0	0	1	216	1016	1	1	889	116
Future Volume (vph)	61	0	110	0	0	1	216	1016	1	1	889	116
Ideal Flow (vphpl)	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900	1900
Storage Length (ft)	0		0	0		0	142		0	0		0
Storage Lanes	0		0	0		0	1		0	0		0
Taper Length (ft)	25			25			25			25		
Lane Util. Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	0.91	0.91	0.91	0.91	0.91
Frt		0.913			0.865						0.983	
Flt Protected		0.982					0.950					
Satd. Flow (prot)	0	1670	0	0	1611	0	1770	5085	0	0	4999	0
Flt Permitted		0.882					0.235				0.939	
Satd. Flow (perm)	0	1500	0	0	1611	0	438	5085	0	0	4694	0
Right Turn on Red			Yes			Yes			Yes			Yes
Satd. Flow (RTOR)		85			108						28	
Link Speed (mph)		30			30			30			30	
Link Distance (ft)		709			271			694			648	
Travel Time (s)		16.1			6.2			15.8			14.7	
Peak Hour Factor	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98	0.98
Adj. Flow (vph)	62	0	112	0	0	1	220	1037	1	1	907	118
Shared Lane Traffic (%)												
Lane Group Flow (vph)	0	174	0	0	1	0	220	1038	0	0	1026	0
Enter Blocked Intersection	No											
Lane Alignment	Left	Left	Right									
Median Width(ft)		0			0			12			12	
Link Offset(ft)		0			0			0			0	
Crosswalk Width(ft)		16			16			16			16	
Two way Left Turn Lane												
Headway Factor	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00	1.00
Turning Speed (mph)	15		9	15		9	15		9	15		9
Number of Detectors	1	2		1	2		1	2		1	2	
Detector Template	Left	Thru										
Leading Detector (ft)	20	100		20	100		20	100		20	100	
Trailing Detector (ft)	0	0		0	0		0	0		0	0	
Detector 1 Position(ft)	0	0		0	0		0	0		0	0	
Detector 1 Size(ft)	20	6		20	6		20	6		20	6	
Detector 1 Type	Cl+Ex	Cl+Ex										
Detector 1 Channel												
Detector 1 Extend (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Queue (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 1 Delay (s)	0.0	0.0		0.0	0.0		0.0	0.0		0.0	0.0	
Detector 2 Position(ft)		94			94			94			94	
Detector 2 Size(ft)		6			6			6			6	
Detector 2 Type		Cl+Ex			Cl+Ex			Cl+Ex			Cl+Ex	
Detector 2 Channel												
Detector 2 Extend (s)		0.0			0.0			0.0			0.0	
Turn Type	Perm	NA		NA		pm+pt	NA		Perm	NA		
Protected Phases		4			8		5	2			6	
Permitted Phases		4			8		2			6		



Lane Group	EBL	EBT	EBR	WBL	WBT	WBR	NBL	NBT	NBR	SBL	SBT	SBR
Detector Phase	4	4		8	8		5	2		6	6	
Switch Phase												
Minimum Initial (s)	5.0	5.0		5.0	5.0		5.0	5.0		5.0	5.0	
Minimum Split (s)	25.3	25.3		25.3	25.3		9.5	24.6		24.6	24.6	
Total Split (s)	36.0	36.0		36.0	36.0		16.0	69.0		53.0	53.0	
Total Split (%)	34.3%	34.3%		34.3%	34.3%		15.2%	65.7%		50.5%	50.5%	
Maximum Green (s)	28.7	28.7		28.7	28.7		11.5	62.4		46.4	46.4	
Yellow Time (s)	3.7	3.7		3.7	3.7		3.5	4.4		4.4	4.4	
All-Red Time (s)	3.6	3.6		3.6	3.6		1.0	2.2		2.2	2.2	
Lost Time Adjust (s)		0.0			0.0		0.0	0.0			0.0	
Total Lost Time (s)		7.3			7.3		4.5	6.6			6.6	
Lead/Lag							Lead			Lag	Lag	
Lead-Lag Optimize?							Yes			Yes	Yes	
Vehicle Extension (s)	3.0	3.0		3.0	3.0		3.0	3.0		3.0	3.0	
Recall Mode	None	None		None	None		None	C-Max		C-Max	C-Max	
Walk Time (s)	7.0	7.0		7.0	7.0			7.0		7.0	7.0	
Flash Dont Walk (s)	11.0	11.0		11.0	11.0			11.0		11.0	11.0	
Pedestrian Calls (#/hr)	0	0		0	0			0		0	0	
Act Effect Green (s)		12.1			12.1		81.1	79.0			65.3	
Actuated g/C Ratio	0.12			0.12			0.77	0.75			0.62	
v/c Ratio	0.70			0.00			0.48	0.27			0.35	
Control Delay	37.6			0.0			7.4	4.7			10.6	
Queue Delay	0.0			0.0			0.0	0.0			0.0	
Total Delay	37.6			0.0			7.4	4.7			10.6	
LOS	D			A			A	A			B	
Approach Delay	37.6							5.2			10.6	
Approach LOS	D							A			B	

Intersection Summary

Area Type: Other

Cycle Length: 105

Actuated Cycle Length: 105

Offset: 100 (95%), Referenced to phase 2:NBTL and 6:SBTL, Start of Green

Natural Cycle: 60

Control Type: Actuated-Coordinated

Maximum v/c Ratio: 0.70

Intersection Signal Delay: 9.7

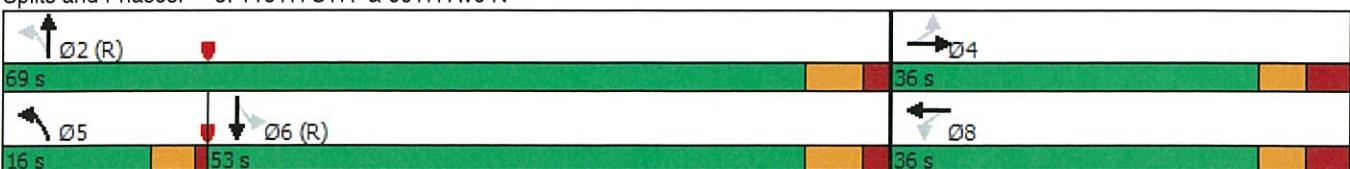
Intersection LOS: A

Intersection Capacity Utilization 73.3%

ICU Level of Service D

Analysis Period (min) 15

Splits and Phases: 3: 113TH STR & 66TH Ave N



Queuing and Blocking Report

Option B-Background Plus Project

10/23/2019

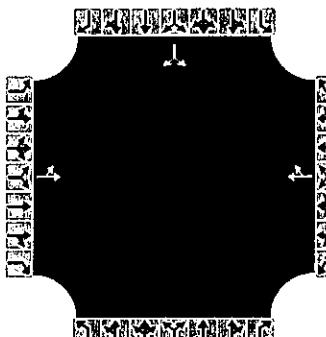
Intersection: 3: 113TH STR & 66TH Ave N

Movement	EB	WB	NB	NB	NB	NB	SB	SB	SB
Directions Served	LTR	LTR	L	T	T	TR	LT	T	TR
Maximum Queue (ft)	187	9	144	173	135	72	237	192	84
Average Queue (ft)	103	1	85	79	65	26	142	100	40
95th Queue (ft)	193	12	149	182	147	76	260	226	105
Link Distance (ft)	651	213		666	666	666	620	620	620
Upstream Blk Time (%)									
Queuing Penalty (veh)									
Storage Bay Dist (ft)				142					
Storage Blk Time (%)				0	2				
Queuing Penalty (veh)				1	4				

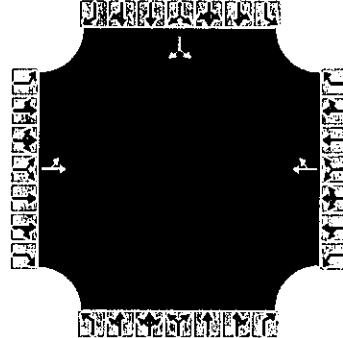
Network Summary

Network wide Queuing Penalty: 5

HCS7 All-Way Stop Control Report

General Information			Site Information																		
Analyst			Intersection																		
Agency/Co.			Jurisdiction																		
Date Performed	10/17/2019			East/West Street			66th Ave North														
Analysis Year	2019			North/South Street			116th St														
Analysis Time Period (hrs)	0.25			Peak Hour Factor			0.75														
Time Analyzed	AM Peak																				
Project Description	Option B-Background +Project																				
Lanes																					
																					
Vehicle Volume and Adjustments																					
Approach	Eastbound			Westbound			Northbound		Southbound												
Movement	L	T	R	L	T	R	L	T	R	L	T	R									
Volume	23	134			28	66				277		14									
% Thrus in Shared Lane																					
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3									
Configuration	LT			TR						LR											
Flow Rate, v (veh/h)	209			125						388											
Percent Heavy Vehicles	2			2						2											
Departure Headway and Service Time																					
Initial Departure Headway, hd (s)	3.20			3.20					3.20												
Initial Degree of Utilization, x	0.186			0.111					0.345												
Final Departure Headway, hd (s)	5.18			4.86					4.96												
Final Degree of Utilization, x	0.301			0.169					0.534												
Move-Up Time, m (s)	2.0			2.0					2.0												
Service Time, ts (s)	3.18			2.86					2.96												
Capacity, Delay and Level of Service																					
Flow Rate, v (veh/h)	209			125					388												
Capacity	695			741					726												
95% Queue Length, Q ₉₅ (veh)	1.3			0.6					3.2												
Control Delay (s/veh)	10.4			8.8					13.5												
Level of Service, LOS	B			A					B												
Approach Delay (s/veh)	10.4			8.8						13.5											
Approach LOS	B			A						B											
Intersection Delay, s/veh LOS	11.8						B														

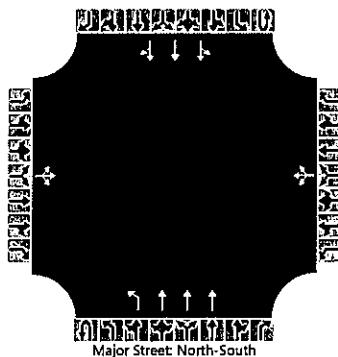
HCS7 All-Way Stop Control Report

General Information			Site Information																			
Analyst			Intersection																			
Agency/Co.			Jurisdiction																			
Date Performed	10/17/2019		East/West Street			66th Ave North																
Analysis Year	2019		North/South Street			116th St																
Analysis Time Period (hrs)	0.25		Peak Hour Factor			0.75																
Time Analyzed	PM Peak																					
Project Description	Option B-Background +Project																					
Lanes																						
																						
Vehicle Volume and Adjustments																						
Approach	Eastbound			Westbound			Northbound		Southbound													
Movement	L	T	R	L	T	R	L	T	R	L	T	R										
Volume	19	67			141	168				92		34										
% Thrus in Shared Lane																						
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3										
Configuration	LT			TR						LR												
Flow Rate, v (veh/h)	115			412						168												
Percent Heavy Vehicles	2			2						2												
Departure Headway and Service Time																						
Initial Departure Headway, hd (s)	3.20			3.20					3.20													
Initial Degree of Utilization, x	0.102			0.366					0.149													
Final Departure Headway, hd (s)	4.89			4.21					5.09													
Final Degree of Utilization, x	0.156			0.482					0.237													
Move-Up Time, m (s)	2.0			2.0					2.0													
Service Time, ts (s)	2.89			2.21					3.09													
Capacity, Delay and Level of Service																						
Flow Rate, v (veh/h)	115			412					168													
Capacity	737			855					708													
95% Queue Length, Q ₉₅ (veh)	0.5			2.7					0.9													
Control Delay (s/veh)	8.8			11.1					9.7													
Level of Service, LOS	A			B					A													
Approach Delay (s/veh)	8.8			11.1						9.7												
Approach LOS	A			B						A												
Intersection Delay, s/veh LOS				10.3						B												

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst		Intersection	
Agency/Co.		Jurisdiction	
Date Performed	10/23/2019	East/West Street	62nd Avenue
Analysis Year	2019	North/South Street	113th Street
Time Analyzed	AM Peak	Peak Hour Factor	0.91
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Option B-Background Plus Project		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	3	0	0	0	3	0
Configuration			LTR				LTR			L	T			LT	T	TR
Volume (veh/h)		36	0	78		0	0	1	0	23	750			1	1204	15
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3	3	3			3		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized																
Median Type Storage		Undivided														

Critical and Follow-up Headways

Base Critical Headway (sec)	6.4	6.5	7.1		6.4	6.5	7.1		5.3				5.3			
Critical Headway (sec)	6.46	6.56	7.16		6.46	6.56	7.16		5.36				5.36			
Base Follow-Up Headway (sec)	3.8	4.0	3.9		3.8	4.0	3.9		3.1				3.1			
Follow-Up Headway (sec)	3.83	4.03	3.93		3.83	4.03	3.93		3.13				3.13			

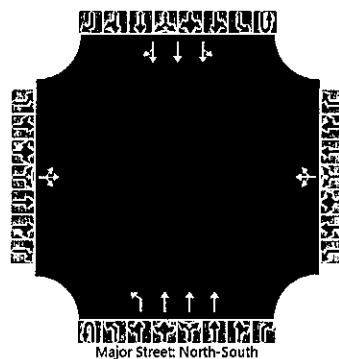
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		125				1			25				1			
Capacity, c (veh/h)		177				501			265				472			
v/c Ratio		0.71				0.00			0.10				0.00			
95% Queue Length, Q ₉₅ (veh)		4.3				0.0			0.3				0.0			
Control Delay (s/veh)		63.5				12.2			20.0				12.6			
Level of Service (LOS)		F				B			C				B			
Approach Delay (s/veh)		63.5				12.2				0.6				0.0		
Approach LOS		F				B										

HCS7 Two-Way Stop-Control Report

General Information		Site Information	
Analyst		Intersection	
Agency/Co.		Jurisdiction	
Date Performed	10/23/2019	East/West Street	62nd Avenue
Analysis Year	2019	North/South Street	113th Street
Time Analyzed	PM Peak	Peak Hour Factor	0.97
Intersection Orientation	North-South	Analysis Time Period (hrs)	0.25
Project Description	Option B-Background Plus Project		

Lanes



Vehicle Volumes and Adjustments

Approach	Eastbound				Westbound				Northbound				Southbound			
	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R
Movement																
Priority		10	11	12		7	8	9	1U	1	2	3	4U	4	5	6
Number of Lanes		0	1	0		0	1	0	0	1	3	0	0	0	3	0
Configuration			LTR				LTR			L	T			LT	T	TR
Volume (veh/h)		21	0	45		0	0	0	0	84	1158			2	887	44
Percent Heavy Vehicles (%)		3	3	3		3	3	3	3					3		
Proportion Time Blocked																
Percent Grade (%)		0				0										
Right Turn Channelized																
Median Type Storage		Undivided														

Critical and Follow-up Headways

Base Critical Headway (sec)		6.4	6.5	7.1		6.4	6.5	7.1		5.3				5.3		
Critical Headway (sec)		6.46	6.56	7.16		6.46	6.56	7.16		5.36				5.36		
Base Follow-Up Headway (sec)		3.8	4.0	3.9		3.8	4.0	3.9		3.1				3.1		
Follow-Up Headway (sec)		3.83	4.03	3.93		3.83	4.03	3.93		3.13				3.13		

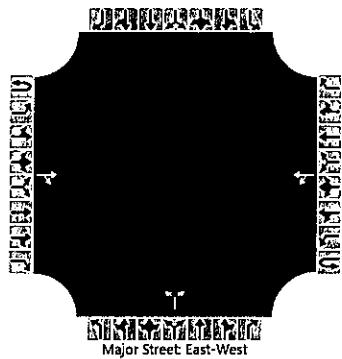
Delay, Queue Length, and Level of Service

Flow Rate, v (veh/h)		68				0			87				2			
Capacity, c (veh/h)			203						406				313			
v/c Ratio			0.34						0.21				0.01			
95% Queue Length, Q ₉₅ (veh)			1.4						0.8				0.0			
Control Delay (s/veh)			31.5						16.2				16.6			
Level of Service (LOS)			D						C				C			
Approach Delay (s/veh)		31.5							1.1				0.1			
Approach LOS		D														

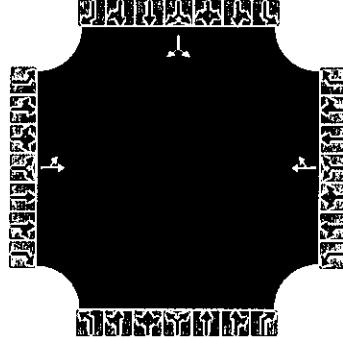
HCS7 Two-Way Stop-Control Report

General Information				Site Information																										
Analyst				Intersection																										
Agency/Co.				Jurisdiction																										
Date Performed	10/23/2019			East/West Street		66th Ave North																								
Analysis Year	2019			North/South Street		Project Access A																								
Time Analyzed	AM Peak			Peak Hour Factor		0.86																								
Intersection Orientation	East-West			Analysis Time Period (hrs)		0.25																								
Project Description	Option B-Background Plus Project																													
Lanes																														
 Major Street: East-West																														
Vehicle Volumes and Adjustments																														
Approach	Eastbound			Westbound			Northbound			Southbound																				
Movement	U	L	T	R	U	L	T	R	U	L	T	R																		
Priority	1U	1	2	3	4U	4	5	6		7	8	9																		
Number of Lanes	0	0	1	0	0	0	1	0	0	1	0	0																		
Configuration				TR		LT				LR																				
Volume (veh/h)			70	0		23	16			0	67																			
Percent Heavy Vehicles (%)						3				3		3																		
Proportion Time Blocked																														
Percent Grade (%)									0																					
Right Turn Channelized																														
Median Type Storage	Undivided																													
Critical and Follow-up Headways																														
Base Critical Headway (sec)					4.1				7.1		6.2																			
Critical Headway (sec)					4.13				6.43		6.23																			
Base Follow-Up Headway (sec)					2.2				3.5		3.3																			
Follow-Up Headway (sec)					2.23				3.53		3.33																			
Delay, Queue Length, and Level of Service																														
Flow Rate, v (veh/h)					27				78																					
Capacity, c (veh/h)					1510				976																					
v/c Ratio					0.02				0.08																					
95% Queue Length, Q ₉₅ (veh)					0.1				0.3																					
Control Delay (s/veh)					7.4				9.0																					
Level of Service (LOS)					A				A																					
Approach Delay (s/veh)				4.4		9.0																								
Approach LOS				A																										

HCS7 Two-Way Stop-Control Report

General Information				Site Information																																								
Analyst				Intersection																																								
Agency/Co.				Jurisdiction																																								
Date Performed	10/23/2019				East/West Street				66th Ave North																																			
Analysis Year	2019				North/South Street				Project Access A																																			
Time Analyzed	PM Peak				Peak Hour Factor				0.66																																			
Intersection Orientation	East-West				Analysis Time Period (hrs)				0.25																																			
Project Description	Option B-Background Plus Project																																											
Lanes																																												
 Major Street: East-West																																												
Vehicle Volumes and Adjustments																																												
Approach	Eastbound				Westbound				Northbound				Southbound																															
Movement	U	L	T	R	U	L	T	R	U	L	T	R	U	L	T	R																												
Priority	1U	1	2	3	4U	4	5	6		7	8	9		10	11	12																												
Number of Lanes	0	0	1	0	0	0	1	0		0	1	0		0	0	0																												
Configuration				TR		LT					LR																																	
Volume (veh/h)			39	0		76	71			0		44																																
Percent Heavy Vehicles (%)						3				3		3																																
Proportion Time Blocked																																												
Percent Grade (%)										0																																		
Right Turn Channelized																																												
Median Type Storage	Undivided																																											
Critical and Follow-up Headways																																												
Base Critical Headway (sec)						4.1				7.1		6.2																																
Critical Headway (sec)						4.13				6.43		6.23																																
Base Follow-Up Headway (sec)						2.2				3.5		3.3																																
Follow-Up Headway (sec)						2.23				3.53		3.33																																
Delay, Queue Length, and Level of Service																																												
Flow Rate, v (veh/h)						115				67																																		
Capacity, c (veh/h)						1538				1004																																		
v/c Ratio						0.07				0.07																																		
95% Queue Length, Q ₉₅ (veh)						0.2				0.2																																		
Control Delay (s/veh)						7.5				8.8																																		
Level of Service (LOS)						A				A																																		
Approach Delay (s/veh)						4.2				8.8																																		
Approach LOS										A																																		

HCS7 All-Way Stop Control Report

General Information			Site Information																			
Analyst			Intersection																			
Agency/Co.			Jurisdiction																			
Date Performed	10/25/2019		East/West Street			Access B/Irving Ave																
Analysis Year	2019		North/South Street			Evergreen Ave																
Analysis Time Period (hrs)	0.25		Peak Hour Factor			0.50																
Time Analyzed	AM Peak																					
Project Description	Option B- Background Plus Project																					
Lanes																						
																						
Vehicle Volume and Adjustments																						
Approach	Eastbound			Westbound			Northbound		Southbound													
Movement	L	T	R	L	T	R	L	T	R	L	T	R										
Volume	0	82			27	1				8		0										
% Thrus in Shared Lane																						
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3										
Configuration	LT			TR						LR												
Flow Rate, v (veh/h)	164			56						16												
Percent Heavy Vehicles	2			2						2												
Departure Headway and Service Time																						
Initial Departure Headway, hd (s)	3.20			3.20					3.20													
Initial Degree of Utilization, x	0.146			0.050					0.014													
Final Departure Headway, hd (s)	4.03			4.10					4.60													
Final Degree of Utilization, x	0.183			0.064					0.020													
Move-Up Time, m (s)	2.0			2.0					2.0													
Service Time, ts (s)	2.03			2.10					2.60													
Capacity, Delay and Level of Service																						
Flow Rate, v (veh/h)	164			56					16													
Capacity	894			877					783													
95% Queue Length, Q ₉₅ (veh)	0.7			0.2					0.1													
Control Delay (s/veh)	7.9			7.4					7.7													
Level of Service, LOS	A			A					A													
Approach Delay (s/veh)	7.9		7.4							7.7												
Approach LOS	A		A							A												
Intersection Delay, s/veh LOS	7.8									A												

HCS7 All-Way Stop Control Report

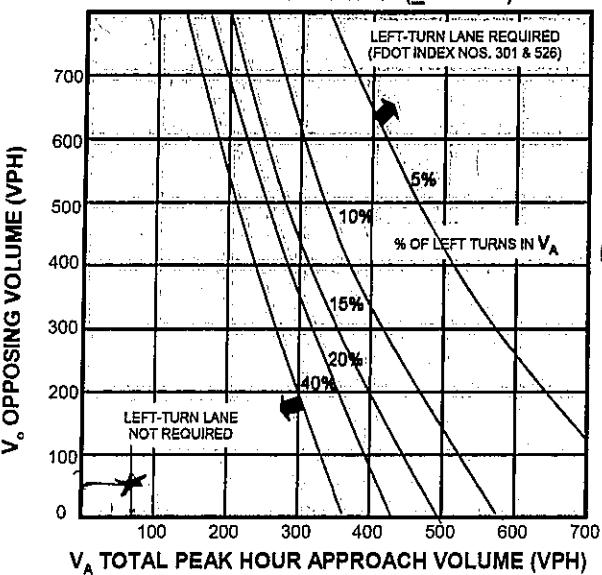
General Information			Site Information																			
Analyst			Intersection																			
Agency/Co.			Jurisdiction																			
Date Performed	10/25/2019		East/West Street			Access B/Irving Ave																
Analysis Year	2019		North/South Street			Evergreen Ave																
Analysis Time Period (hrs)	0.25		Peak Hour Factor			0.56																
Time Analyzed	PM Peak																					
Project Description	Option B- Background Plus Project																					
Lanes																						
Vehicle Volume and Adjustments																						
Approach	Eastbound			Westbound			Northbound		Southbound													
Movement	L	T	R	L	T	R	L	T	R	L	T	R										
Volume	0	54			92	8				2		0										
% Thrus in Shared Lane																						
Lane	L1	L2	L3	L1	L2	L3	L1	L2	L3	L1	L2	L3										
Configuration	LT			TR						LR												
Flow Rate, v (veh/h)	96			179						4												
Percent Heavy Vehicles	2			2						2												
Departure Headway and Service Time																						
Initial Departure Headway, hd (s)	3.20			3.20					3.20													
Initial Degree of Utilization, x	0.086			0.159					0.003													
Final Departure Headway, hd (s)	4.11			3.99					4.71													
Final Degree of Utilization, x	0.110			0.198					0.005													
Move-Up Time, m (s)	2.0			2.0					2.0													
Service Time, ts (s)	2.11			1.99					2.71													
Capacity, Delay and Level of Service																						
Flow Rate, v (veh/h)	96			179					4													
Capacity	877			903					765													
95% Queue Length, Q ₉₅ (veh)	0.4			0.7					0.0													
Control Delay (s/veh)	7.6			8.0					7.7													
Level of Service, LOS	A			A					A													
Approach Delay (s/veh)	7.6			8.0						7.7												
Approach LOS	A			A						A												
Intersection Delay, s/veh LOS	7.8						A															

NCHRP 279



LINCKS & ASSOCIATES, INC.

GRAPH 2A. LEFT-TURN LANE WARRANTS –
TWO-LANE FACILITIES (\leq 40 MPH)



NOTE: Left-turn lane not required when intersection of V_A and V_o is below the curve corresponding to the % of left turns in V_A .

AM - OPTION A
Project Access/66th Avenue NORTH

$$V_A = 666$$

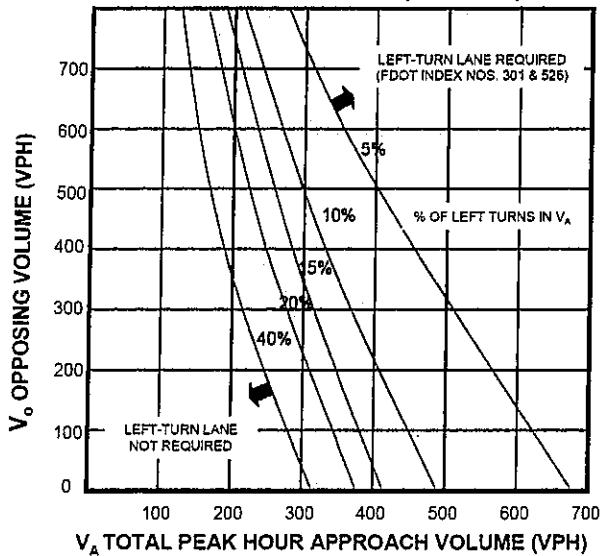
$$V_o = 70$$

$$\% \text{ left turn in } V_A =$$

$$50\% = 70$$

WBL turn lane not warranted

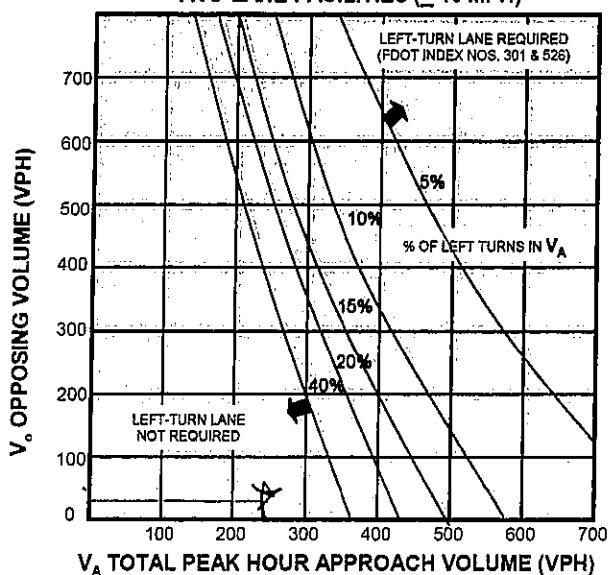
GRAPH 2B. LEFT-TURN LANE WARRANTS –
TWO-LANE FACILITIES (45-50 MPH)



NOTE: Left-turn lane not required when intersection of V_A and V_o is below the curve corresponding to the % of left turns in V_A .

Graphs 2A & 2B Source: National Cooperative Highway Research Program, Report No. 279.

**GRAPH 2A. LEFT-TURN LANE WARRANTS –
TWO-LANE FACILITIES (< 40 MPH)**



NOTE: Left-turn lane not required when intersection of V_A and V_o is below the curve corresponding to the % of left turns in V_A .

Am - option A
project access/66th Ave
A NORTH

$$VA = 239$$

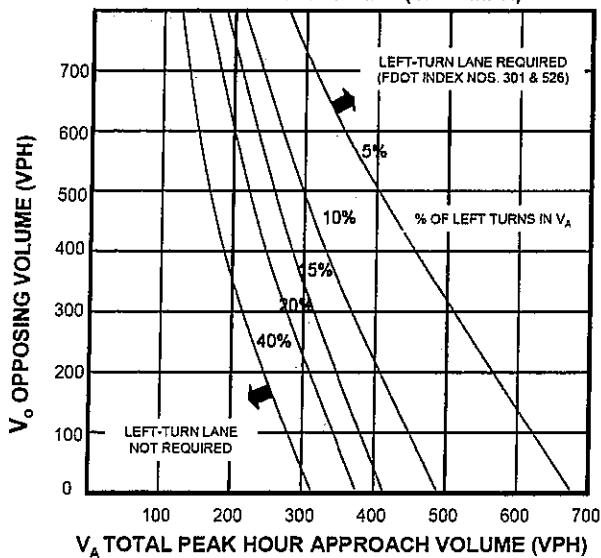
$$VO = 39$$

% of left turns in VA =

$$16B / 239 = 70$$

WBL turn lane not warrant ad

**GRAPH 2B. LEFT-TURN LANE WARRANTS –
TWO-LANE FACILITIES (45-50 MPH)**



NOTE: Left-turn lane not required when intersection of V_A and V_o is below the curve corresponding to the % of left turns in V_A .

Graphs 2A & 2B Source: National Cooperative Highway Research Program, Report No. 279.

AM - option B

Project Access A / 66th Avenue N

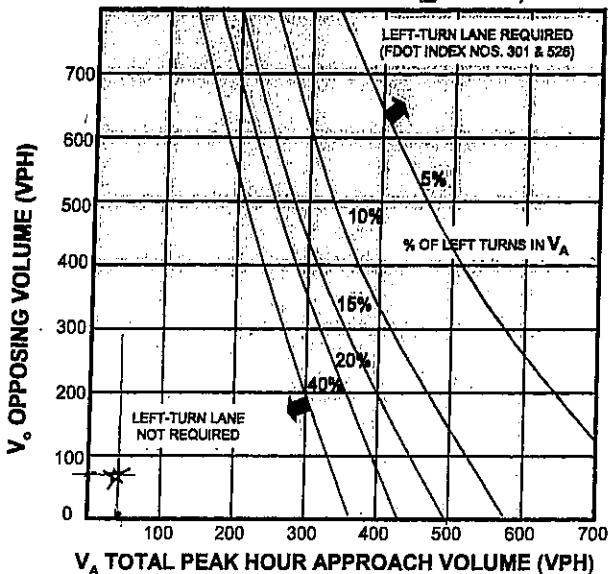
$$VA = 39$$

$$VO = 70$$

$$\% \text{ of Left turns in } VA \\ \frac{23}{39} = 59$$

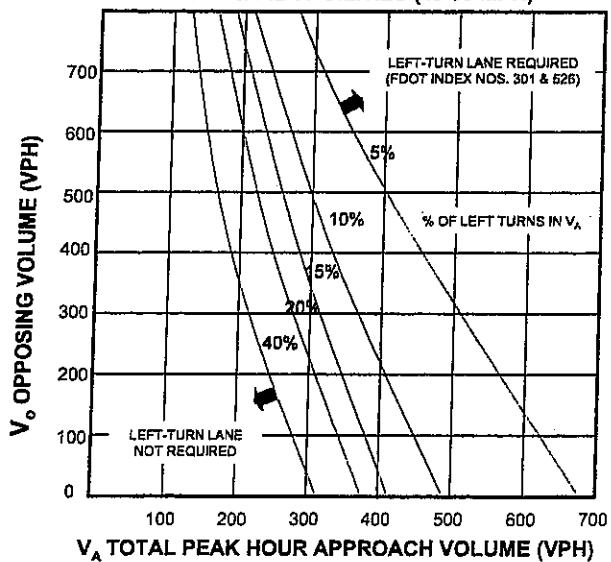
WBL is not warranted

GRAPH 2A. LEFT-TURN LANE WARRANTS –
TWO-LANE FACILITIES (≤ 40 MPH)



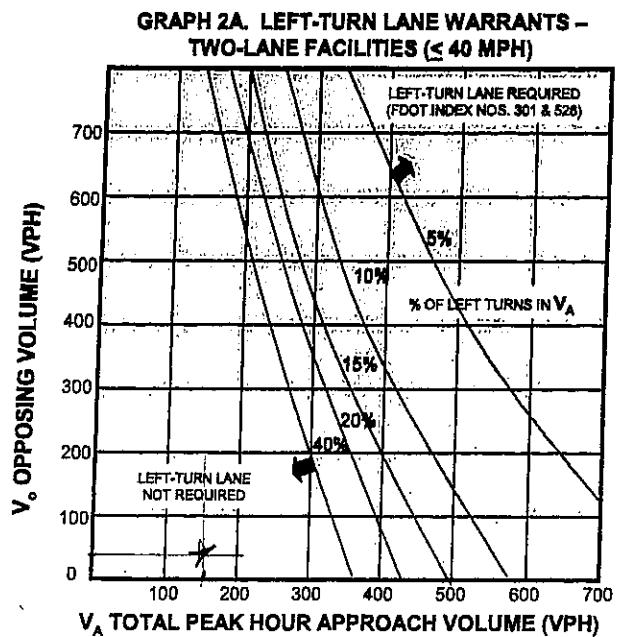
NOTE: Left-turn lane not required when intersection of V_A and V_o is below the curve corresponding to the % of left turns in V_A .

GRAPH 2B. LEFT-TURN LANE WARRANTS –
TWO-LANE FACILITIES (45-50 MPH)

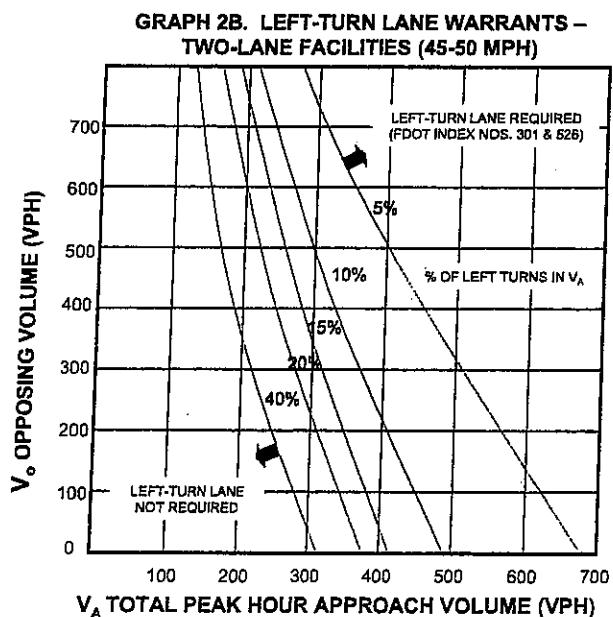


NOTE: Left-turn lane not required when intersection of V_A and V_o is below the curve corresponding to the % of left turns in V_A .

Graphs 2A & 2B Source: National Cooperative Highway Research Program, Report No. 279.



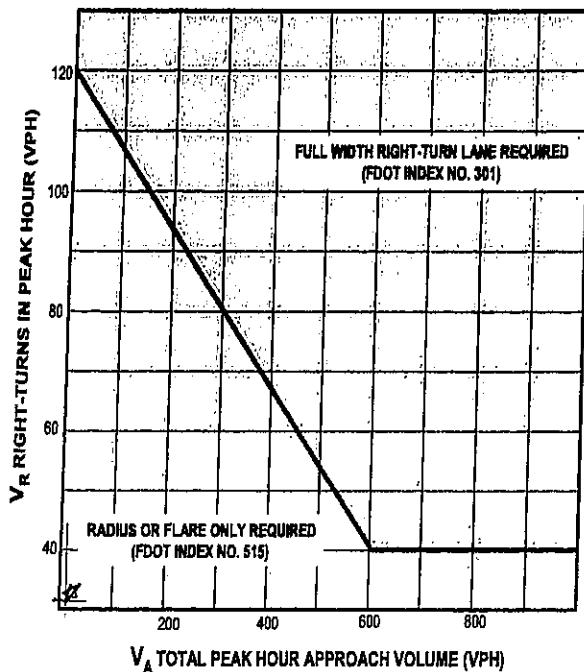
$V_A = 147$
 $V_O = 39$
% of left turns in V_A
 $\frac{76}{147} = 52$
WBL is not warranted.



NOTE: Left-turn lane not required when intersection of V_A and V_O is below the curve corresponding to the % of left turns in V_A .

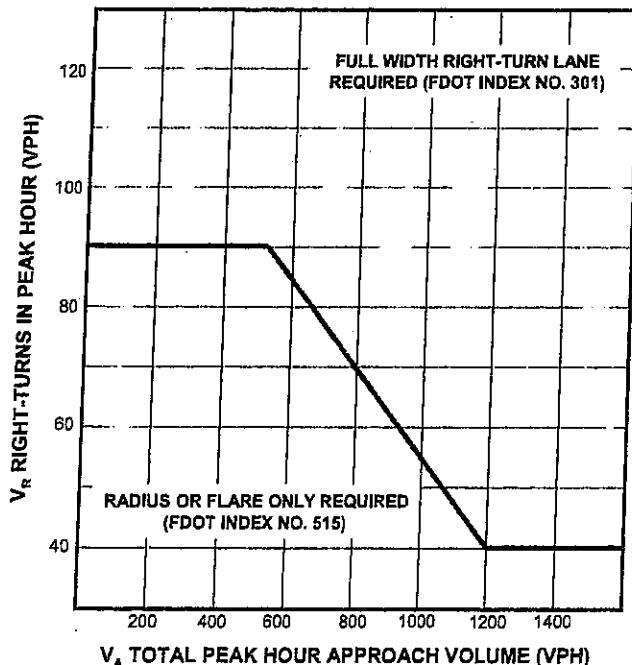
Graphs 2A & 2B Source: National Cooperative Highway Research Program, Report No. 279.

GRAPH 1A. RIGHT-TURN LANE WARRANTS - TWO-LANE FACILITIES



Am - option B
 Project access B/
 Evergreen Avenue.
 $\sqrt{A} = 28$
 $VR = 1$
 WBR is not
 warranted.

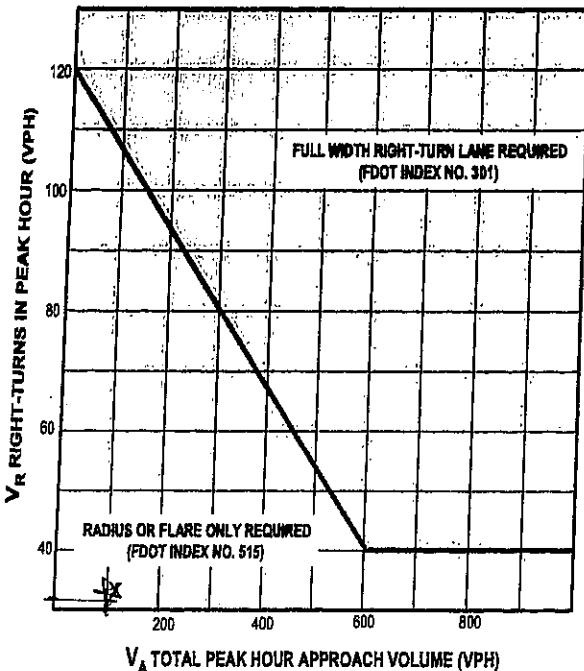
NOTE: For posted speeds at or under forty-five (45) mph, peak hour right turns greater than forty (40) VPH, and total peak hour approach less than 300 VPH, adjust right turn volumes. Adjust peak hour right turns = peak hour right turns - twenty (20).

GRAPH 1B. RIGHT-TURN LANE WARRANTS
FOUR- OR SIX-LANE FACILITIES

NOTE: For application on high speed highways.

Graphs 1A & 1B Source: National Cooperative Highway Research Program, Report No. 279.

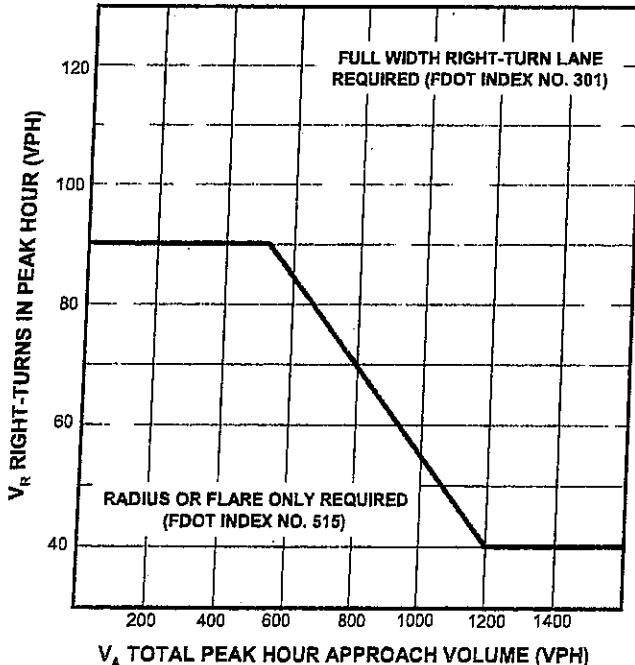
GRAPH 1A. RIGHT-TURN LANE WARRANTS - TWO-LANE FACILITIES



PM - option B
Project Access B
EVERgreen Avenue
 $\sqrt{A} = 100$
 $VR = 8$
WBR is not warranted

NOTE: For posted speeds at or under forty-five (45) mph, peak hour right turns greater than forty (40) VPH, and total peak hour approach less than 300 VPH, adjust right turn volumes. Adjust peak hour right turns = peak hour right turns - twenty (20).

GRAPH 1B. RIGHT-TURN LANE WARRANTS
FOUR- OR SIX-LANE FACILITIES



NOTE: For application on high speed highways.

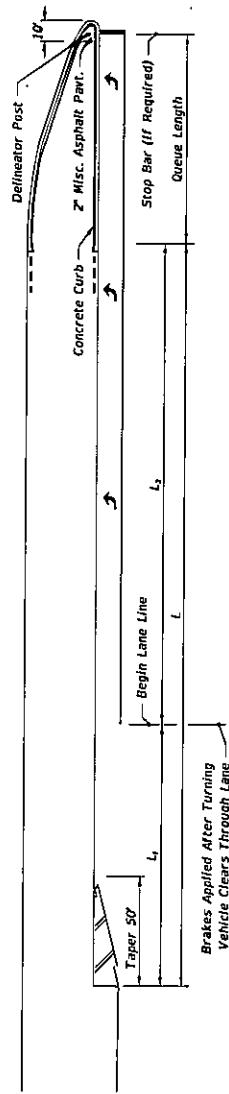
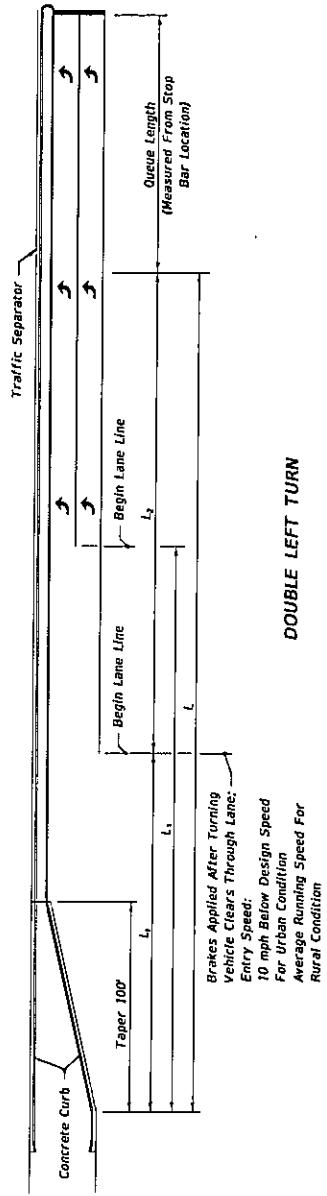
Graphs 1A & 1B Source: National Cooperative Highway Research Program, Report No. 279.

FDOT EXHIBIT 212-1



LINCKS & ASSOCIATES, INC.

MEDIAN TURN LANES MINIMUM DECELERATION LENGTHS



MEDIAN TURN LANES					
URBAN CONDITIONS			RURAL CONDITIONS		
Design Speed (mph)	Entry Speed (mph)	Clearance Distance L_1 (ft.)	Brake To Stop Distance L_2 (ft.)	Clearance Distance L_3 (ft.)	Brake To Stop Distance L_4 (ft.)
35	25	70	75	145	110
40	30	80	75	155	120
45	35	85	100	185	135
50	40/44	105	135	240	160
55	48	125	—	—	225
60	52	145	—	—	260
65	55	170	—	—	290

NOT TO SCALE

EXHIBIT 212-1
01/01/2018